

AN ELASTIC-PLASTIC FINITE ELEMENT  
ANALYSIS OF NOTCHED ALUMINUM PANELS

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# NAVAL POSTGRADUATE SCHOOL

## Monterey, California



# THESIS

AN ELASTIC-PLASTIC FINITE ELEMENT ANALYSIS  
OF NOTCHED ALUMINUM PANELS

by

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March 1981

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T199335





REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1. REPORT NUMBER	2. GOVT ACCESSION NO.	3. RECIPIENT'S CATALOG NUMBER
4. TITLE (and Subtitle) An Elastic-Plastic Finite Element Analysis of Notched Aluminum Panels		5. TYPE OF REPORT & PERIOD COVERED Master's Thesis March 1981
		6. PERFORMING ORG. REPORT NUMBER
7. AUTHOR(s) Michael John Kaiser		8. CONTRACT OR GRANT NUMBER(s)
9. PERFORMING ORGANIZATION NAME AND ADDRESS Naval Postgraduate School Monterey, California 93940		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS
11. CONTROLLING OFFICE NAME AND ADDRESS Naval Postgraduate School Monterey, California 93940		12. REPORT DATE March 1981
		13. NUMBER OF PAGES 155 pages
14. MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office)		15. SECURITY CLASS. (of this report) Unclassified
		15a. DECLASSIFICATION/DOWNGRADING SCHEDULE
16. DISTRIBUTION STATEMENT (of this Report) Approved for public release; distribution unlimited		
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report)		
18. SUPPLEMENTARY NOTES		
19. KEY WORDS (Continue on reverse side if necessary and identify by block number) Finite element analysis, stress, stress concentration factor, notched panels, residual stress, plastic zone, ADINA, extrapolation, elastic-plastic stress.		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) Finite element, elastic and plastic analyses of various alumi- num panels, containing holes and notches, were conducted for com- parison with photoelastic experimental results. A FORTRAN IV program, ADINA (Automatic Dynamic Incremental Nonlinear Analysis), was used for both linear and nonlinear analyses. Mesh refinements were used for each panel and the monotonically convergent results were extrapolated using Richardson's method. Stresses were		



Item 20 (contd).

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of Notched Aluminum Panels

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Submitted in partial fulfillment of the  
requirements for the degree of

MASTER OF SCIENCE IN AERONAUTICAL ENGINEERING

from the

NAVAL POSTGRADUATE SCHOOL  
March 1981

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Travel  
Lodge  
- 1

## ABSTRACT

Finite element, elastic and plastic analyses of various aluminum panels, containing holes and notches, were conducted for comparison with photoelastic experimental results. A FORTRAN IV program, ADINA (Automatic Dynamic Incremental Nonlinear Analysis), was used for both linear and nonlinear analyses. Mesh refinements were used for each panel and the monotonically convergent results were extrapolated using Richardson's method. Stresses were locally smoothed from the Gauss integration points to the nodal points. Eight noded, isoparametric elements were used throughout. Modification to an ADINA preprocessor program, also coded in FORTRAN IV, was made for use with a VERSATEC plotter.

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## SYMBOLS AND ABBREVIATIONS

ADINA	Automatic Dynamic Incremental Nonlinear Analysis
$b$	Half width of strip
CPU	Central processor unit
$E$	Young's Modulus of Elasticity
$E_t$	Strain hardening tangent modulus
FEA	Finite element analysis
JCL	Job control language
$K_T$	Stress concentration factor referenced to reduced cross-section $\sigma/\sigma_n$
MVS	Multiple virtual storage
$n$	Ramberg-Osgood exponent
$O(h^m)$	Order of the discretization error
$r$	Radius of hole or notch
VM	Virtual machine
$\beta$	Ramberg-Osgood coefficient
$\epsilon$	General representation for strain
$\lambda$	Non-dimensional size parameter $\lambda=r/b$
$\nu$	Poisson's Ratio of transverse strain
$\sigma$	General representation for stress
$\sigma_\theta$	Principle stress in $\theta$ direction (hoop stress)
$\sigma_r$	Principle stress in radial direction
$\sigma_n$	Nominal stress in reduced cross-section
$\sigma_\infty$	Far-field stress
$\sigma_Y$	Yield stress by 0.2% offset method



## ACKNOWLEDGEMENT

I would like to thank all the people who assisted me in my education at the Naval Postgraduate School.

In particular, I would like to thank Professor G. H. Lindsey for his guidance in completing this work. I would also like to thank Professor G. Cantin for introducing me to the finite element method; Mr. Bob Besel and his staff for their support; and the staff of the W. R. Church Computer Center of the Naval Postgraduate School for their assistance.

I thank my wife and family for their sacrifices while I have been in pursuit of my career.





## I. INTRODUCTION

Development of on-board fatigue monitoring systems for Naval aircraft have made it possible to record extensive structural loading data in flight. The strain gages used in such a system must be located away from stress concentration areas to prevent their fatigue; however, these areas are of the greatest interest in analyzing and predicting fatigue life of the structure. Understanding the relationship between nominal, far-field stresses and local stresses in critical areas thus becomes vitally important. Recent experimental investigations into the effect of uniform, far-field loads on stress concentration areas have been made at the Naval Postgraduate School (NPS) using photoelastic techniques [Refs. 1, 2 and 3]. These experiments involved loading 7075-T6 aluminum into both the elastic and plastic regions, as well as measurements of residual stresses resulting from plastic yielding.

Finite element analyses (FEA) of the aluminum panels used in the experiments of Stenstrom [Ref. 1] were conducted. The panels used in the experiments of Engle [Ref. 2] and Stuart [Ref. 3] have similar geometry. The finite element programs available at NPS were surveyed and ADINA [Ref. 4] was chosen for its proven ability to produce the nonlinear analyses required for plastic



yielding of aluminum. To provide increased accuracy, each panel was modeled using two meshes. The results obtained for the coarse and fine meshes were extrapolated to a final result using the Richardson extrapolation technique [Refs. 5 and 6].

Along with ADINA, a preprocessor program, PSAP1 [Ref. 7], was used to verify mesh connectivity prior to analysis by ADINA. PSAP1 provides a graphical output of the finite element mesh and was coded for the CALCOMP plotter installed at NPS prior to 1978. For this thesis PSAP1 was adapted for use with the VERSATEC plotter now installed at NPS.

The stress-strain material properties of the 7075-T6 aluminum actually used to make the panels had to be established to provide an accurate material model for use with ADINA. Material testing was conducted to establish the Young's Modulus ( $E$ ), Poisson's Ratio ( $\nu$ ), yield stress ( $\sigma_y$ ), strain hardening modulus ( $E_t$ ) and the Ramberg-Osgood coefficients  $\beta$  and  $n$ .

Comparisons were made to other works, in addition to the experiments conducted at NPS. The initial analysis involved a comparison of FEA to the results of Howland [Ref. 8], for a circular hole in a finite strip, to validate the methods used. A comparison of FEA to plane stress, slip-line theory, for rigid-perfectly-plastic material was also included as a validation for the plastic analyses.



## II. MATERIAL PROPERTY TESTING OF 7075-T6 ALUMINUM

The elastic and plastic material properties of the aluminum panels were established by tensile tests of uni-axial specimens made from the same mill run. The specimens were manufactured and tested according to current ASTM standards [Ref. 9]. MICRO-MEASUREMENTS, EA-13-125AD-120, precision strain gages with a temperature compensated bridge circuit were used on all specimens. Transverse gage sensitivity errors were corrected according to the manufacturer's recommendations [Ref. 10]. Critical cross-section measurements were made with a micrometer.

### A. TESTS FOR AXIAL LOADING

Initial tests of the two-gaged specimen, Fig. 1, in the MTS testing machine indicated a significant bending moment was being produced by the 30,000 lb GRIFF grips. To investigate this problem further, tests were conducted on both the MTS and RIEHLE test machines with a five-gaged specimen shown in Fig. 2. The results of these axial loading tests, shown in Table I, verified that the GRIFF grips on the MTS test machine do not give axial loading. An inspection of the gripped region on the specimen indicated that the jaws of the grip were not applying a uniformly distributed force and thereby induced a bending



moment by off-axis loading as shown in Fig. 2. The grips on the RIEHLE test machine gripped evenly and a uniform strain distribution resulted as seen in Table I.

## B. CHARACTERISTICS OF 7075-T6 ALUMINUM PANELS

The following characteristic properties were determined from the four specimens tested.

### 1. Young's Modulus (E)

Tests were conducted using the specimen shown in Fig. 3 on the MTS test machine with 10,000 lb INSTRON grips, which gripped the specimen evenly. The results of testing three specimens are shown in Tables II to IV. Linear regression in the elastic range of all the test results determined a Young's Modulus of  $10.12 \times 10^6$  psi, with a correlation coefficient of 0.9996.

### 2. Poisson's Ratio ( $\nu$ )

Tests were conducted using the specimen shown in Fig. 1 on the RIEHLE test machine with 10,000 lb RIEHLE grips. The results are tabulated in Table V. Linear regression of these results in the elastic region determined Poisson's Ratio to be 0.3256 with a correlation coefficient of 0.99996.

### 3. Yield Stress and Strain Hardening Modulus

These values, required for ADINA's bi-linear material model, were determined graphically using the data from Tables III and IV. Plastic region data in





Table II is not reliable because of excessive creep encountered during that test.

0.2% offset yield stress,  $\sigma_y = 76,000$  psi

strain hardening modulus,  $E_t = 566,000$  psi

The graphical fit of these values to the test data can be seen in Fig. 4.

#### 4. Ramberg-Osgood Coefficients

The Ramberg-Osgood equation for elastic-plastic stress-strain characterization is given by:

$$\epsilon = \frac{\sigma}{E} + \beta \left( \frac{\sigma}{E} \right)^n \quad (1)$$

where:

$\epsilon$  = strain

$\sigma$  = stress

$E$  = Young's modulus.

The  $\beta$  and  $n$  coefficients were determined graphically from the data of Table IV, by the method given by Rivello [Ref. 11]. The data in Table IV gave the following values which are the best fit to the combined test data

$$\beta = 1.479 \times 10^{43}$$

$$n = 21.58$$

The graphical fit of these values, in Eq. (1), with the test data is also shown in Fig. 4.



### III. MODIFICATION TO GRAPHICAL PREPROCESSOR

The use of a graphical preprocessor program, such as PSAP1, is vital in detecting mesh errors that may otherwise go unnoticed. Establishing the proper node locations and element geometry prior to analysis for a complex code such as ADINA is of utmost importance.

#### A. PSAP1 MODIFICATIONS

The program PSAP1 was originally coded in FORTRAN IV for use on the NPS IBM 360/370 installation with the CALCOMP Model 765 drum plotter. The CALCOMP system once installed at NPS used the +Y axis as the unlimited plotting axis, see Fig. 5. The entire plotting logic in PSAP1 uses this orientation of axes to allow multiple plots in a continuous strip. With the VERSATEC Model 8222A electrostatic plotter now installed at NPS, the +X axis becomes the unlimited plotting axis, shown in Fig. 5. To avoid an extensive recoding of PSAP1 for use with the VERSATEC plotter, a simple coordinate transformation of the plot was made in a limited number of short subroutines. First, all installation dependent plotting calls used in PSAP1 were identified. These involved seven plotter functions for which new subroutines were coded.



<u>Function</u>	<u>New Subroutine</u>
Initialize Plotter	CALCMP
Move Plotter Pen	CALPLT
Letter on Plot	NOTATE
Number on Plot	CALNUM
Determine Current Pen Location	CALWH
Draw a Line	CALINE
Stop Plotter	PSTOP

The subroutines listed above merely rotates the plot to coincide with the VERSATEC axis orientation and retain all features originally in PSAP1. Since all plotter hardware code is now isolated in these seven subroutines, future adaptations to other plotting systems is simplified. To provide documentation of this update to PSAP1, a complete listing of the new program is provided in Appendix D.

#### B. USE OF PSAP1

Previous use of PSAP1 on the IBM 360 system necessitated use of a load module since PSAP1 took over one minute of CPU time to compile. With the new IBM 3033 system compilation requires eight seconds; however, use of a load module or disk stored source code is still recommended since PSAP1 contains roughly 2,500 lines of code. Appendix A contains sample JCL to use PSAP1 on the IBM 3033 MVS system. With minimal effort PSAP1 could also be set up



for use on the IBM VM/370 system. The user's manual for PSAP1 is in Ref. 7. In addition to a mesh plot, PSAP1 provides a listing of the node coordinates, element connectivity and several key input values used in execution of ADINA. This information provides a useful check of the input data.





#### IV. FINITE ELEMENT ANALYSIS (FEA)

##### A. DESCRIPTION OF MESHES USED

###### 1. Length to Width Ratios and Boundary Conditions

Initial finite element models of specimens had length to width ratios near one, as used by Garske [Ref. 12] for his FEA, but they did not provide the desired uniform distribution at the loading boundary. Specimen length to width ratios of 3-5 were used by Armen, Pifko and Levine [Ref. 13] in their FEA and by Stenstrom [Ref. 1] in his photoelastic experiments. The criteria established to determine uniform boundary stress distribution was uniformity in nodal displacements along the loaded edge as discussed by Segerlind [Ref. 14]. In the models used for FEA in this thesis, nodal displacements were uniform to within 0.1%, and the resulting stress distribution was uniform axially to within 0.1% at the panel ends. In all cases two-dimensional, eight noded, isoparametric elements were used. These higher order elements cannot be loaded in an "intuitive" manner as discussed by Zienkiewicz [Ref. 15, p. 223]. Figure 6 shows the nodal loading required to obtain a uniform surface load.



## 2. Element Meshes

Two meshes were developed for each panel analyzed. A reasonable effort was made to keep element corner angles as close to  $90^{\circ}$  as possible to reduce the effect of element distortion discussed by Hopkins and Gifford [Ref. 16]. All meshes modeled a quarter of the actual panel by using the two axes of symmetry as is common practice in FEA. The step from course to fine element meshes was made so that each element in the course mesh was subdivided into four smaller elements of the same type. Such a mesh subdivision can be expected to give monotonic convergence of results, Cook [Ref. 17], and allow extrapolation to results of an infinitely fine mesh. Figures 7 through 13 illustrate the element meshes used in this analysis as plotted by PSAP1.

### B. COMPUTATIONAL PROCEDURES

#### 1. Using ADINA

Once the mesh has been developed, input data is prepared in accordance with the ADINA user's manual [Ref. 4]. This same set of data is then used as input for PSAP1 to check for errors and provide a graphical display of the element mesh. After preprocessing by PSAP1, the data is entered into ADINA for analysis. In the case of linear analysis, two types of stress output may be specified, nodal point or Gauss integration point. Nodal point output



can be computed for up to eight node point stresses for each element. Since 2x2 Gauss integration was used, four Gauss point stresses were computed for each element. The 2x2 Gauss integration is recognized as the most efficient integration order for this type of analysis [Ref. 15, p. 284]. The linear analysis used an isotropic linear elastic material model (MODEL "1" in Ref. 4) which required input of E and  $\nu$  material properties. The nonlinear analysis allows only Gauss point stress outputs and uses a bilinear elastic-plastic material model, with von Mises yield condition and isotropic strain hardening (MODEL "8" in Ref. 4).

For static analyses ADINA uses a time function method to apply loads in steps. Linear analysis loading was accomplished in a single step to a nominal value of 3,000 lbs load. Nonlinear analysis loads were applied in ten steps to a maximum value, matching the experimental loads, and then unloaded to zero in ten steps to obtain residual stresses. The stress output from ADINA is a listing of nodal or Gauss point stresses for each element. Since the only area of interest in this analysis was the distribution of stresses along the reduced cross-section, no large post-processing program was developed or used. All final computations using ADINA output data were accomplished on a HEWLETT-PACKARD 9830A calculator, using short programs coded in BASIC. If more extensive stress



distribution information were desired, some form of automated post-processing would be necessary to reduce the computational workload. At a minimum, nodal stress outputs by ADINA must be averaged to obtain unique values of stress at nodes shared by more than one element.

## 2. Richardson Extrapolation

The use of course and fine meshes allows extrapolation to an infinitely fine mesh as discussed earlier. Richardson extrapolation [Ref. 5] was used in this analysis where:

$$\sigma_{\text{extrap}} = \frac{\sigma_C (h_F)^m - \sigma_F (h_C)^m}{h_F^m - h_C^m} \quad (2)$$

where

$\sigma_{\text{extrap}}$  = extrapolated solution

$\sigma_C$  = solution obtained with  $h=h_C$

$\sigma_F$  = solution obtained with  $h=h_F$

$h_C$  = linear dimension of course element

$h_F$  = linear dimension of fine element

$m$  = 2 (for this analysis)

The exponent  $m$  is determined by the order of the discretization error  $O(h^m)$ . Since  $h$  represents the length of an element the element area is represented  $h^2$ . In a two dimensional problem such as this  $O(h^m)$  is of the order of  $h^2$ , the area of an element. In the mesh refinement scheme





used  $h_F = \frac{1}{2} h_C$  or  $\frac{h_F}{h_C} = \frac{1}{2}$ . Equation (2) can be rewritten

$$\sigma_{\text{extrap}} = \frac{\sigma_C \left(\frac{h_F}{h_C}\right)^2 - \sigma_F \left(\frac{h_C}{h_C}\right)^2}{\left(\frac{h_F}{h_C}\right)^2 - \left(\frac{h_C}{h_C}\right)^2} \quad (3)$$

thus

$$\sigma_{\text{extrap}} = \frac{\sigma_F - \frac{1}{4} \sigma_C}{\frac{3}{4}} \quad (4)$$

Equation (4) then becomes the relation to obtain extrapolated stresses from coarse and fine mesh results in a two dimensional analysis. Better extrapolations can be obtained by using three or more refined meshes, but the computational effort increases significantly.

### 3. Optimal Stress Locations and Local Smoothing

It is generally accepted that the most accurate sampling points for stresses are the Gauss integration points within the element [Ref. 15, p. 281, and Ref 18]. In this analysis, the nodal points are of the greatest interest; thus a technique of local smoothing must be applied to the integration point stresses to obtain nodal stresses as reported by Hinton and Campbell [Ref. 19]. The formulation of this local smoothing technique for ADINA elements is developed in Appendix B. The nodal values obtained must then be averaged if shared by two or more elements.



#### 4. Computational Times

Because of the extremely large size of ADINA (about 17,000 lines of code in the NPS version) and the out of core solver, it does not adapt well to time sharing systems. Using the IBM 360 system at NPS, ADINA required 31 user defined overlays to create a manageable load module in about 30 minutes of CPU time. With the new IBM 3033 MVS system at NPS, ADINA is compiled without overlays in about one minute. When using a load module, the program execution took less than 2 minutes CPU time on the IBM 3033.

In addition to ADINA, the preprocessor (PSAP1) and post-processing techniques involve considerable time and effort. Figure 14 is a flow chart of the computational procedure used in this analysis. An example of the JCL to use ADINA in load module form on the IBM 3033 MVS system with use of the mass storage facilities is shown in Appendix C.



## V. RESULTS OF ANALYSIS

### A. CIRCULAR HOLES IN LINEAR MATERIAL

The FEA results for a circular hole in a finite width strip were used to validate the elastic computational procedure discussed earlier. The results of Howland [Ref. 8] were compared to both the Gauss point smoothed results and the nodal output results in Fig. 15. The stress concentration factor  $\sigma/\sigma_\infty$  is referenced to the far-field stress.

The smoothed results give the best match to the results of Howland at the edge of the hole, and the only significant variation between the two FEA methods occurs within the first 0.25 inches from the edge. In order to obtain the 0.25 inch stress value for the coarse mesh, in the Gauss point smoothed result, a midside node value had to be obtained by the averaging method discussed in Appendix

B. The linear distribution of smoothed stresses along the sides of the element, [Ref. 19], appears to produce a less accurate result in this area of extreme stress gradient, when compared to ADINA's nodal output result. This tendency was noted in all cases; however, the peak stress values from smoothed results consistently gave better correlation with other investigators [Ref. 20].

A circular hole with  $\lambda=0.25$  was also analyzed and compared to the experimental results of Stenstrom [Ref. 1]



along with an interpolation of Howland's results. The  $\sigma_{\theta}$  experimental data correlates well with the FEA results; however, the  $\sigma_r$  experimental data shows significant variation between 0.125 and 0.375 inches from the edge of the hole, as seen in Fig. 16.

## B. OPPOSITE U NOTCHES IN LINEAR MATERIAL

The results for linear analysis are presented in non-dimensional stress concentration form; however, the normalizing stress changes. For U notches  $K_T$  is the stress concentration factor referenced to the theoretical, nominal stress ( $\sigma_n$ ) in the reduced cross-section where  $\sigma_n = \text{Load/Area of Reduced Cross-Section}$ .

### 1. Shallow Notch Panel

The FEA results plotted with the experimental data of Stenstrom are shown in Fig. 17. Once again both FEA results are shown and the variation for the two methods occur within 0.25 inches from the notch edge; however, there was less variation than was seen in the circular hole analyses.

For this panel the experimental data appear to be uniformly below the FEA results for  $\sigma_{\theta}$ . The  $\sigma_r$  data shows significant variation at the 0.625 inch point but follows the proper trend within 0.5 inches from the notch edge. According to data collected by Peterson [Ref. 20], this notch geometry should yield a maximum  $K_T = 2.74$ . The





Gauss point smoothed results matched this value exactly. Stuart [Ref. 3] reported a  $K_T = 2.69$  for this same notch geometry, with a standard deviation of 0.187 in the 14 samples he measured by photoelastic methods. Early photoelastic work by Frocht [Ref. 21] determined  $K_T = 2.7$  for this notch geometry. The FEA results appear to be in good agreement with other investigators, for this notch geometry.

## 2. Deep Notch Panel

Results of the FEA for a deep notch were plotted with Stenstrom's experimental data in Fig. 18. The results of the two FEA methods again diverge within 0.5 inches from the notch edge. FEA stress values at the 0.25 inch point have spread farther apart in this case since the stress gradient is very severe at that point. The experimental data correlates well for both  $\sigma_\theta$  and  $\sigma_r$ ; however, the maximum experimental  $\sigma_\theta$  is considerably lower with a  $K_T = 3.83$ . Results reported by Stuart for this notch geometry was a  $K_T = 4.05$  with a standard deviation of 0.219 for 14 specimens measured by photoelastic methods. Frocht [Ref. 22] reported a photoelastic  $K_T = 3.9$  for this notch geometry, but concluded that the result was 5-10% low, giving a corrected range of  $K_T$  from 4.1 to 4.3. The Gauss smoothed FEA result gave a  $K_T = 4.24$  which compares well to an empirical relation given by Peterson [Ref. 20] for  $r/d < 0.25$ .



$$K_T = (1 - \frac{2t}{D}) (0.78 + 2.243\sqrt{t/r}) \left[ 0.993 + 0.18 \frac{2t}{D} - 1.06 (\frac{2t}{D})^2 + 1.71 (\frac{2t}{D})^3 \right] \quad (5)$$

where

t = notch depth (3.9375)

r = notch radius (0.625)

d = minimum width (15.625)

D = maximum width (23.5)

Inserting the values above into Eq. (5) gives a  $K_T = 4.26$ , which is 1/2% above the FEA result. Other FEA results reported by Armen, Pifko and Levin [Ref. 13] and Griffis [Ref. 23], using linear strain triangle (LST) elements, produced  $K_T$  values within 5% of those produced by use of Eq. (5). The rectangular elements used in this analysis are known to give better results than LST elements as noted by Clough [Ref. 24]. It is clear that the FEA results obtained for this notch are in good agreement with other works.

### C. OPPOSITE U NOTCHES IN NONLINEAR MATERIAL

The analysis for loading into the plastic region of the 7075-T6 aluminum was made using the bilinear material model discussed earlier. The loads used were selected to match those used in the experiments of Stenstrom; thus allowing direct comparison. The strains obtained in those experiments were used to solve for stresses by use of the Prandtl-Reuss plastic flow equations.



## 1. Shallow Notch Panel

The results of FEA for the three load cases, 60,000, 65,000 and 70,000 lbs are presented along with the experimental results in Figs. 19 through 21. The  $\sigma_\theta$  results compare well although no trend for peak  $\sigma_\theta$  stress away from the notch edge is shown in the experimental data. In all cases the FEA determined the peak  $\sigma_\theta$  stress to occur near the yield boundary, and the gradient of the  $\sigma_\theta$  stress to fall off dramatically in the plastic zone. This characteristic behavior of the  $\sigma_\theta$  stress was reported by other investigators [Refs. 13 and 23] using FEA on 2024-T3 aluminum. Plane elastic-plastic stress distributions reported by Frocht [Ref. 25] show similar trends. The experimental data also shows a marked change in the gradient of  $\sigma_\theta$  stress within the plastic region. The growth of this plastic region is approximated using the FEA results for this notch in Figs. 22 through 24.

Experimental data for the  $\sigma_r$  stress distribution matches the FEA results closely except at the notch edge where the measured  $\sigma_r$  does not go to zero as it should. The characteristic peak value of  $\sigma_r$  near the plastic boundary as seen in the FEA results is also shown by the test data.

The FEA residual stress computed upon unloading from the three load cases are shown in Figs. 25 through 30. The characteristic distributions of the  $\sigma_\theta$  residual stress



agrees with those reported by others [Refs. 25, 26 and 27]. The experimental residual stress distributions reported by Stenstrom show similar trends but significant variations when compared to the FEA results.

## 2. Deep Notch Panel

Three load cases were computed to plastic loading levels; however, only limited experimental results are available for this notch as seen in Fig. 31. The 30,000 lb load is just at the onset of yield in the notch root area. Limited residual experimental data [Ref. 3] was available for comparison in Figs. 32 and 33 which are plots of the residual  $\sigma_{\theta}$  and  $\sigma_r$  stress distributions as a result of the three loading cases. The 30,000 lb load has caused yielding in a small region at the root of the notch as seen in Fig. 34. Figures 35 and 36 illustrate that the plastic zone does not grow to the extent it did in the shallow notch. The stress gradients are very severe in the deep notch and as a result the 0.25 inch sampling points used in the FEA may be useful in only showing gross trends close to the notch edge. The trends appear to be much the same as in the shallow notch, with the peak  $\sigma_{\theta}$  and  $\sigma_r$  stresses occurring near the yield boundary. The yield boundaries shown in Fig. 31 are approximations based on qualitative analysis of the finite element results. The  $\sigma_{\theta}$  experimental data for the 30,000 lb load case correlates well with the peak stress, again





appearing low as it did in the linear analysis. The residual  $\sigma_\theta$  stress distribution in Fig. 32 follow much the same trends as seen in the shallow notch but with much higher gradients within the first 0.5 inches from the notch. The FEA results for the residual  $\sigma_r$  stresses shown in Fig. 33 indicate a limitation in the element size used since the  $\sigma_r$  value of the notch edge does not return to zero as it should. Because of this problem the data may be questionable for showing proper trends in the first 0.5 inch from the notch edge.

### 3. Rigid-Perfectly Plastic Panel

A stress distribution for the theoretical material used in slip-line theory was desired. By using ADINA's bilinear material model such a material could be approximated reasonably well. For this analysis a Young's Modulus (E) of  $10^{26}$  psi was used to model perfect rigidity. The strain hardening modulus ( $E_t$ ) was set to zero to model perfect plasticity. Poisson's ratio ( $\nu$ ) was 0.4999999, as close to 0.5 as the computer would allow. The  $E = 10^{26}$  also represents a computer limit in approaching an infinitely large E. Figure 37 illustrates the results obtained and compares them to a slip-line solution. The results are normalized to the arbitrary 73,000 psi yield stress used in the analysis. The  $\sigma_\theta$  values obtained agree exactly with slip-line theory. Conversely the  $\sigma_r$  results do not reflect the same values as slip-line theory, but do show



a similar trend. The growth of the plastic zone obtained is shown in Figs. 38 through 40.



## VI. CONCLUSIONS AND RECOMMENDATIONS

The results obtained from the FEA have proven useful in determining the validity of experimental data gathered by photoelastic techniques. The FEA results varied by less than 1% when compared to published analytical results and handbook values. Experimental data from Stenstrom's photoelastic work correlated well with the FEA results. The primary exception was the residual stress experimental values, which varied significantly from the FEA results. The variation may be in transforming the residual strains measured photoelastically into stresses for comparison with the FEA results, since ADINA only provides a stress output. Limitations of ADINA's bilinear material model, initially considered severe, do not appear to have hampered this investigation. A possible exception is the residual analyses where the transition region from elastic to plastic strains becomes especially important. The Gauss point smoothed results gave the best correlations at the edge singularities in all cases; however, due to the limitations noted at 0.25 inches from the edge, the results at that specific point may not be as accurate for this method. The nodal output results gave consistently higher stress values at the edge singularities. Because



of the severe stress gradients near the deep notch analyzed the use of a finer element mesh near the notch would probably produce better results.

The effort involved in developing two meshes such as those used in this thesis is considerable. An automatic mesh generation capability would reduce the workload and allow experimentation with several types of element meshes.

ADINA proved to be a useful and powerful program, as expected, but something simpler and less awkward to use may be all that is required for two dimensional analysis. Such a system is already in use at NPS but does not offer non-linear capabilities. If use of ADINA is to be continued in this type of investigation, a post-processing program should be adapted. There are programs available to post-process ADINA data at NPS [Refs. 28 and 29] but they would require modifications to work with two dimensional analyses and the VERSATEC plotter.

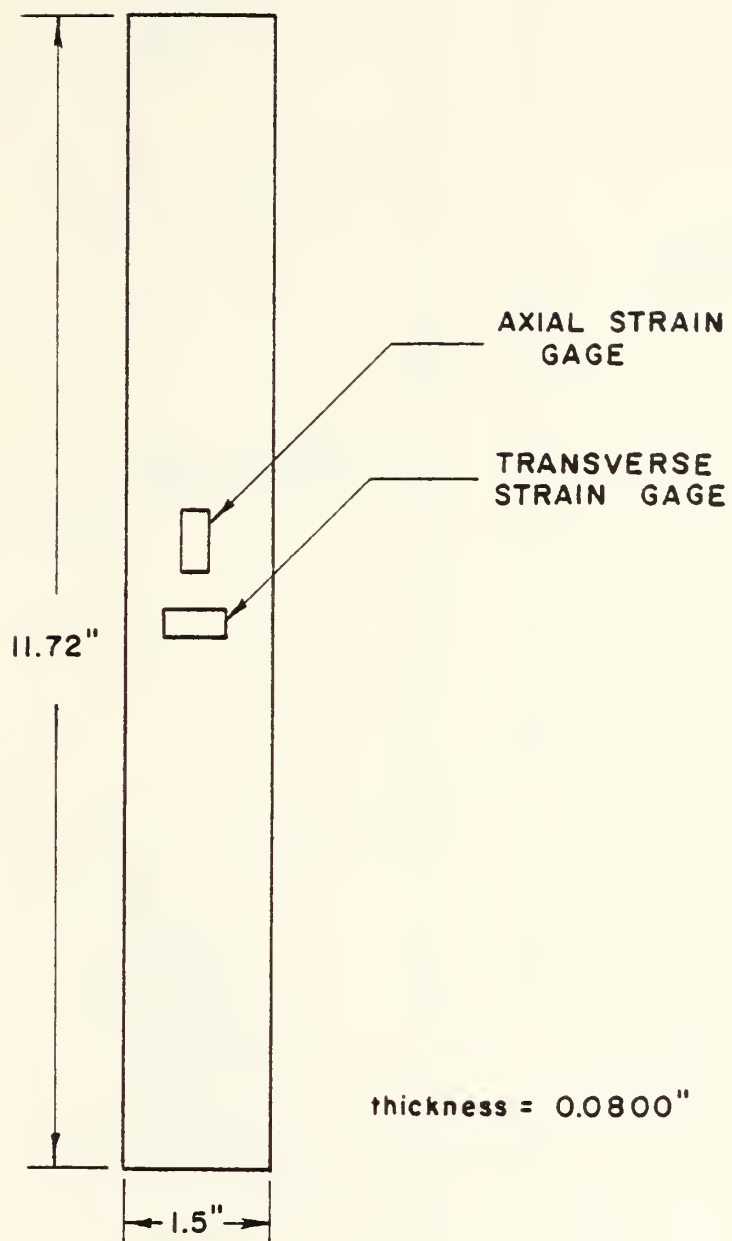
Standardized material property testing would ease the inevitable task of obtaining basic material properties for use in analysis or experiments. Some form of automatic data collection with use of the MTS testing machine would allow testing of a larger sample population and provide statistically more accurate information.





FIGURE 1

2 GAGE SPECIMEN





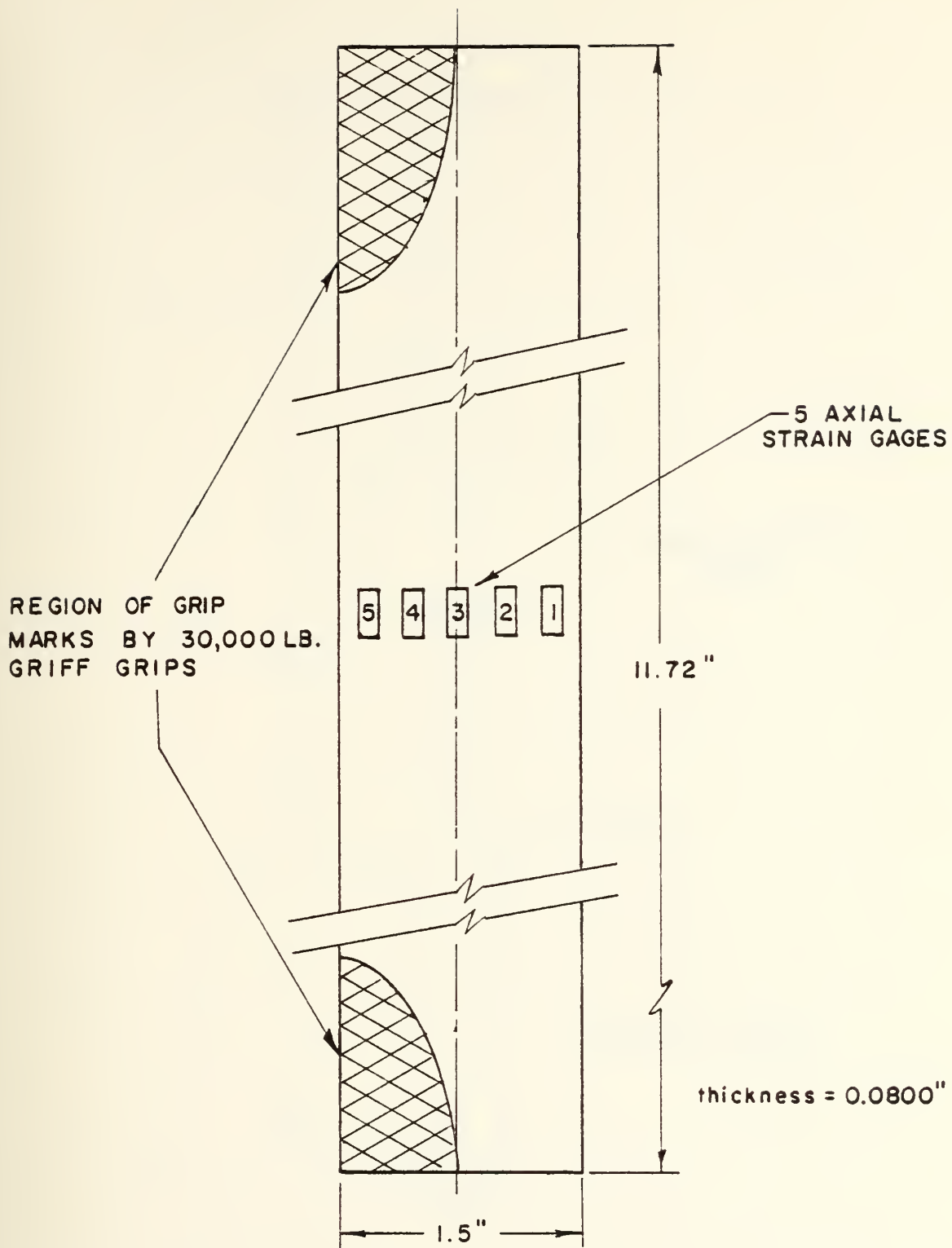


FIGURE 2

5 GAGE SPECIMEN



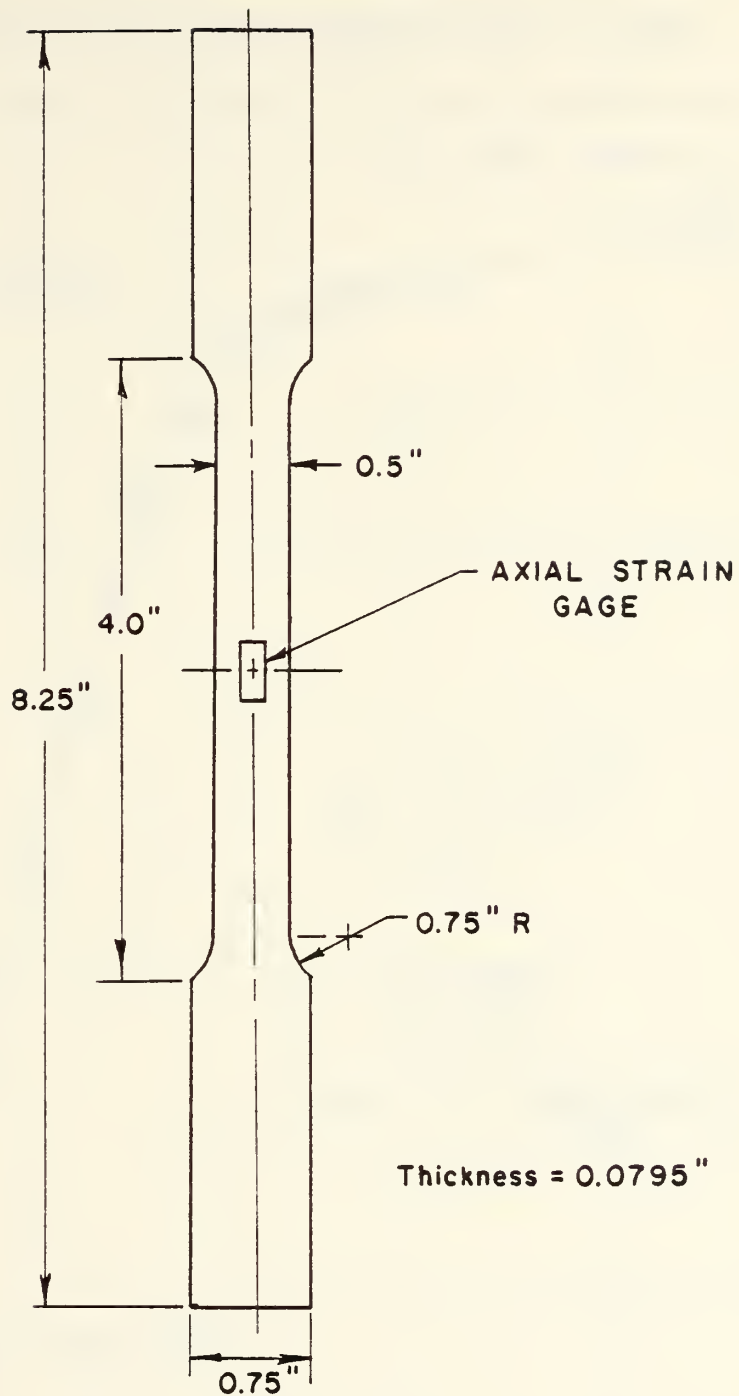
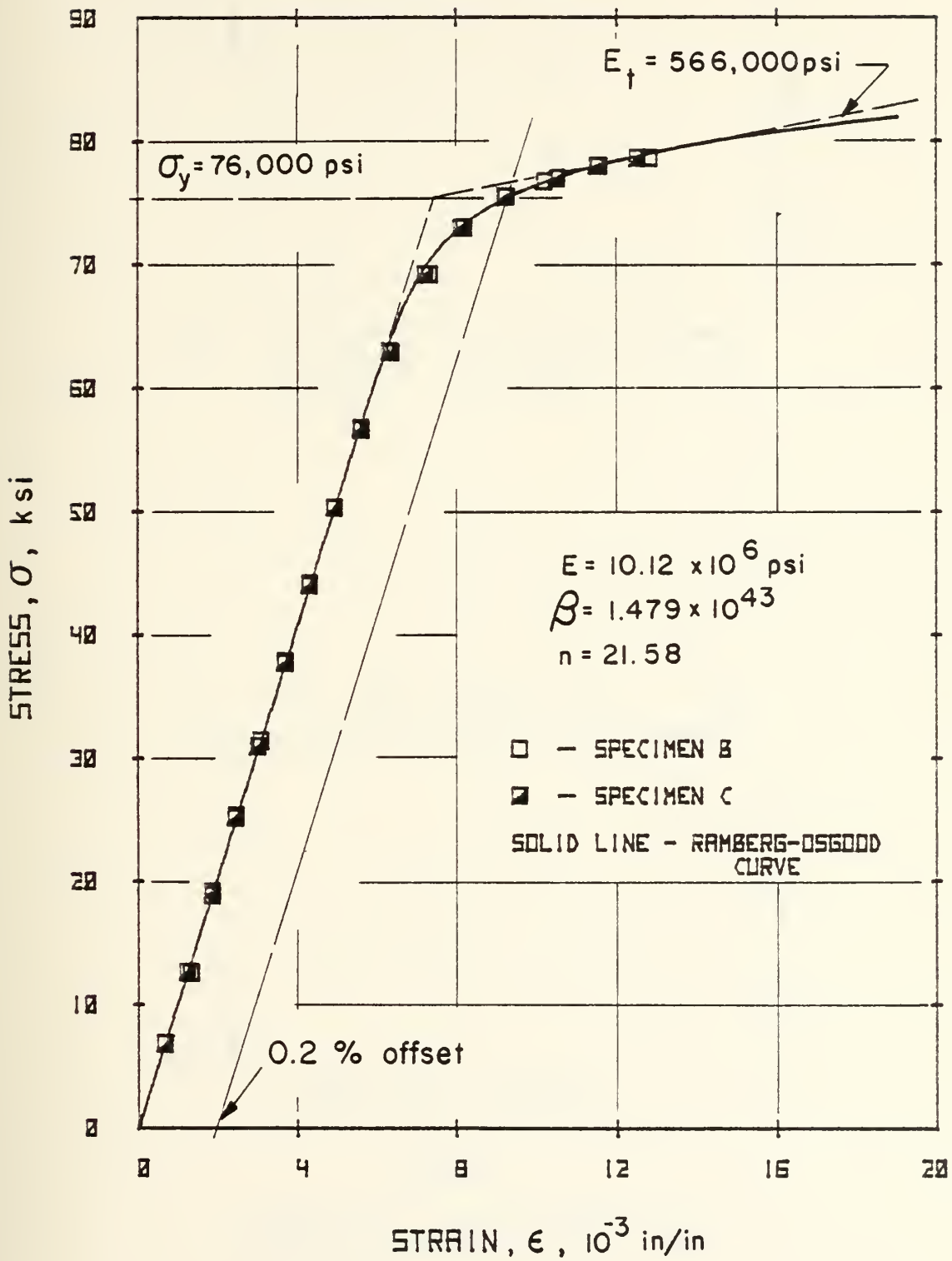


FIGURE 3  
1 GAGE SPECIMEN



FIGURE 4

7075-T6 ALUMINUM STRESS-STRAIN CURVE







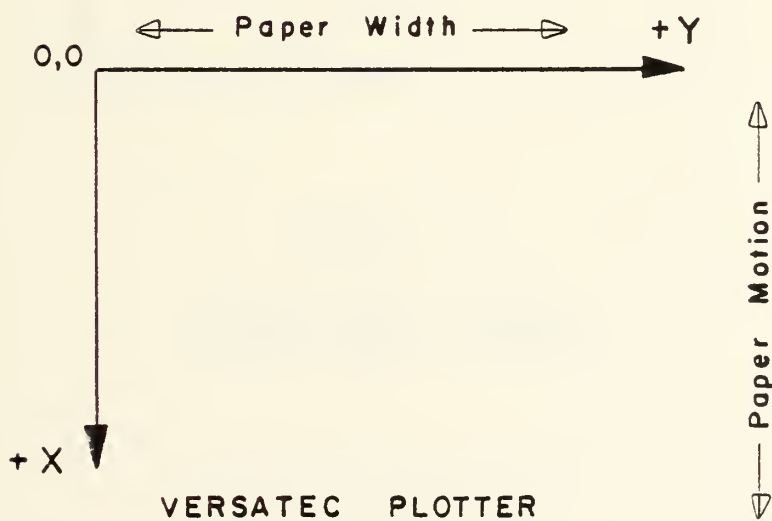
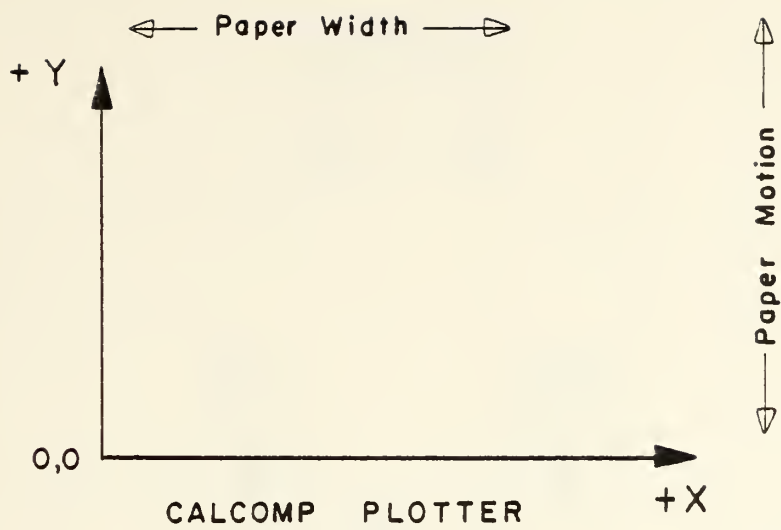


FIGURE 5

CALCOMP AND VERSATEC PLOTTER AXES



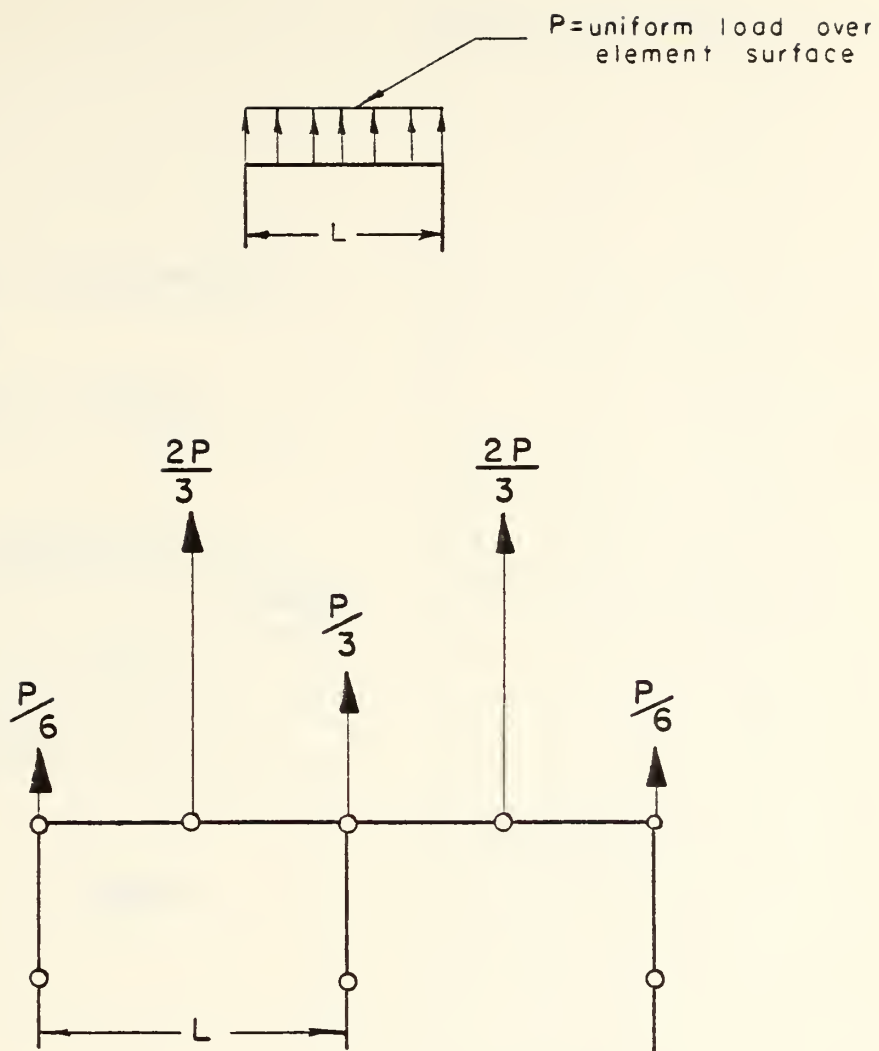


FIGURE 6

NODAL LOADING DIAGRAM



28 ELEMENTS (ISOPARAMETRIC)

111 NODES

192 DEGREES OF FREEDOM

DIMENSIONS

$\lambda$	W	L	RADIUS
0.2	5"	25"	1"
0.25	4.0625"	20"	1"

$$\lambda = \frac{\text{RADIUS}}{W}$$

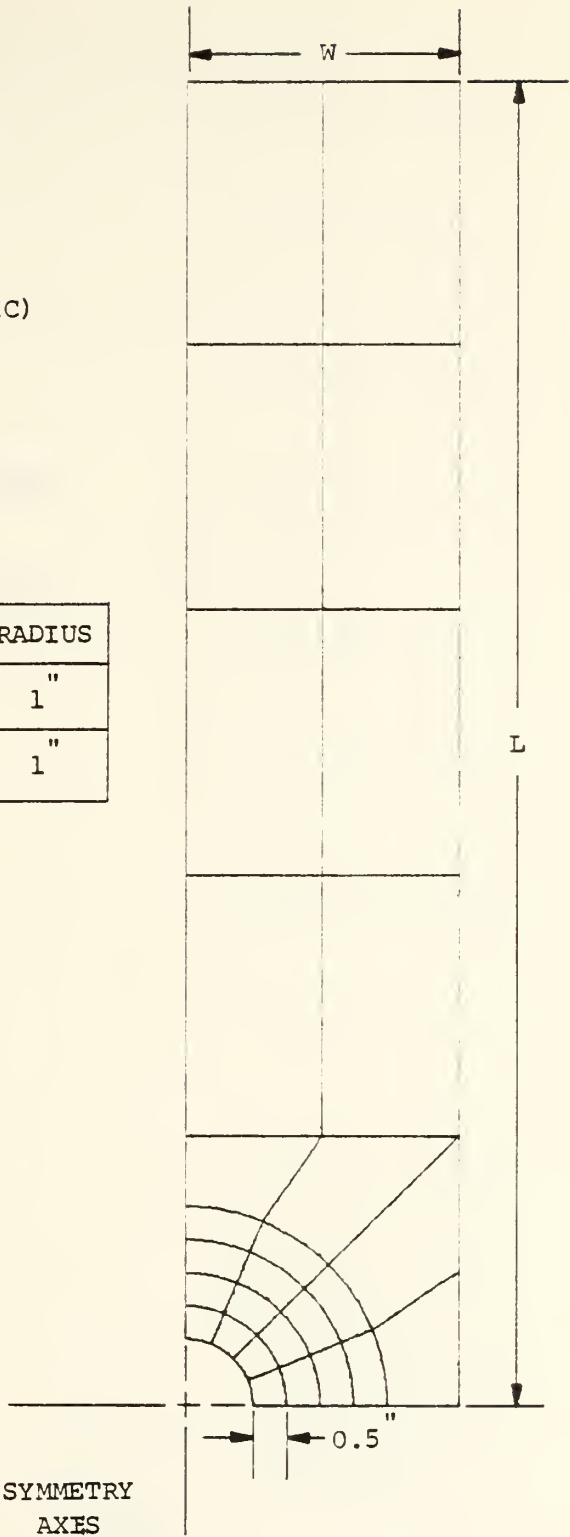


FIGURE 7

COURSE MESH FOR CIRCULAR HOLES



A SUBDIVISION OF COURSE MESH

112 ELEMENTS (ISOPARAMETRIC)

389 NODES

720 DEGREES OF FREEDOM

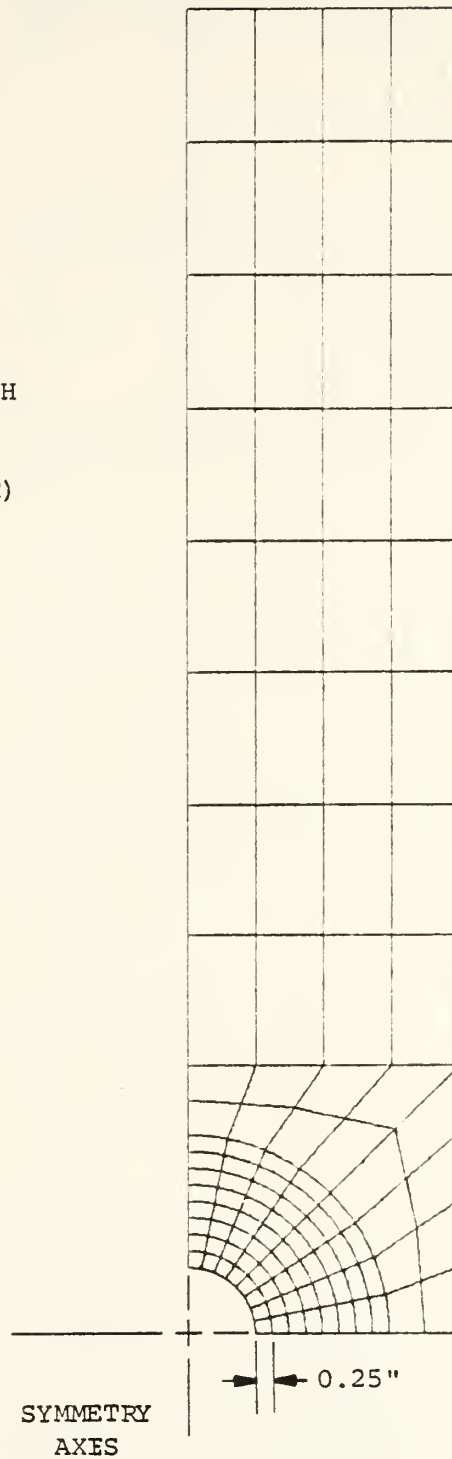


FIGURE 8

FINE MESH FOR CIRCULAR HOLES





FIGURE 9

## COURSE MESH FOR SHALLOW NOTCH

60 ELEMENTS  
(ISOPARAMETRIC)  
219 NODES  
404 DEGREES OF  
FREEDOM

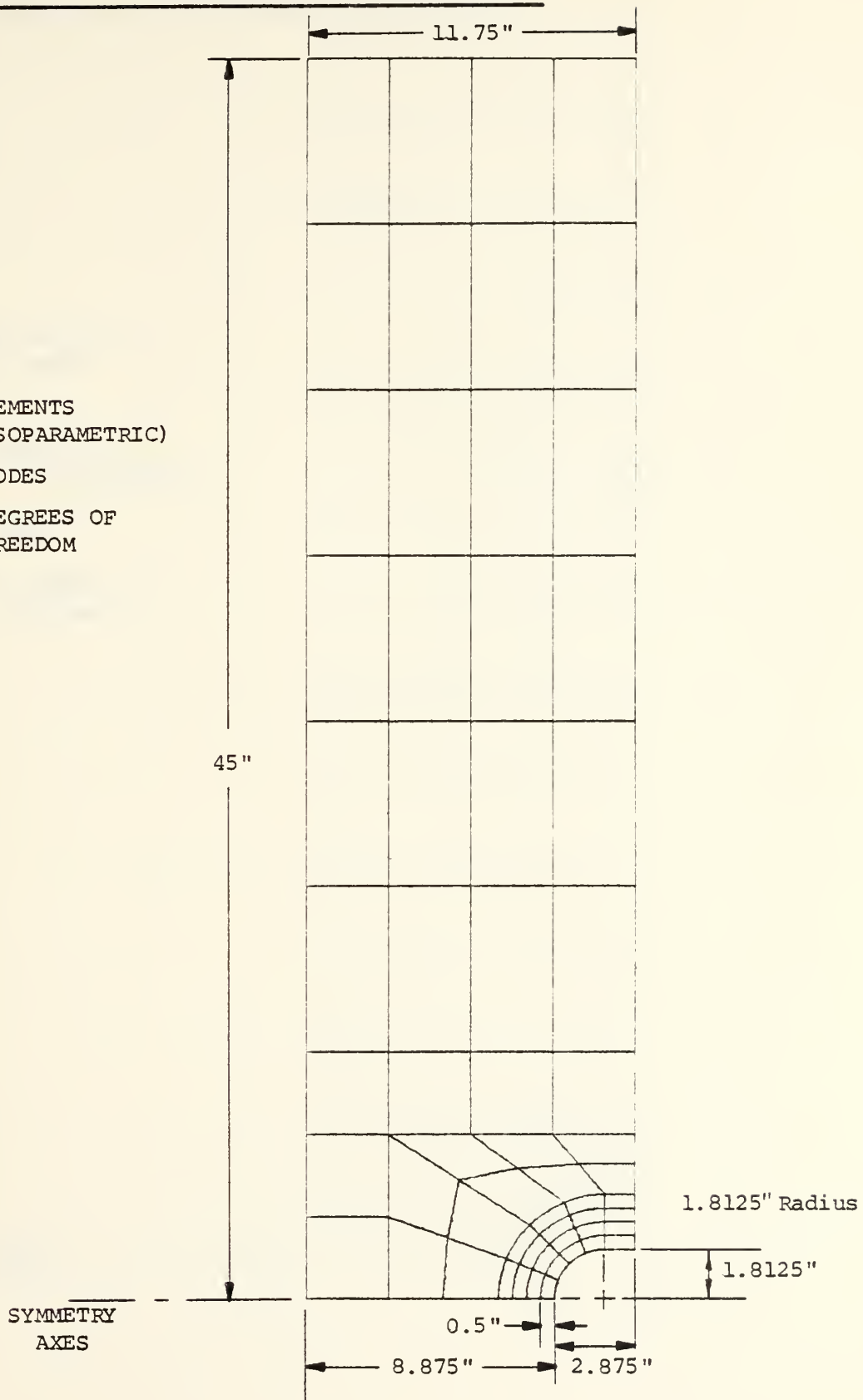




FIGURE 10 FINE MESH FOR SHALLOW NOTCH

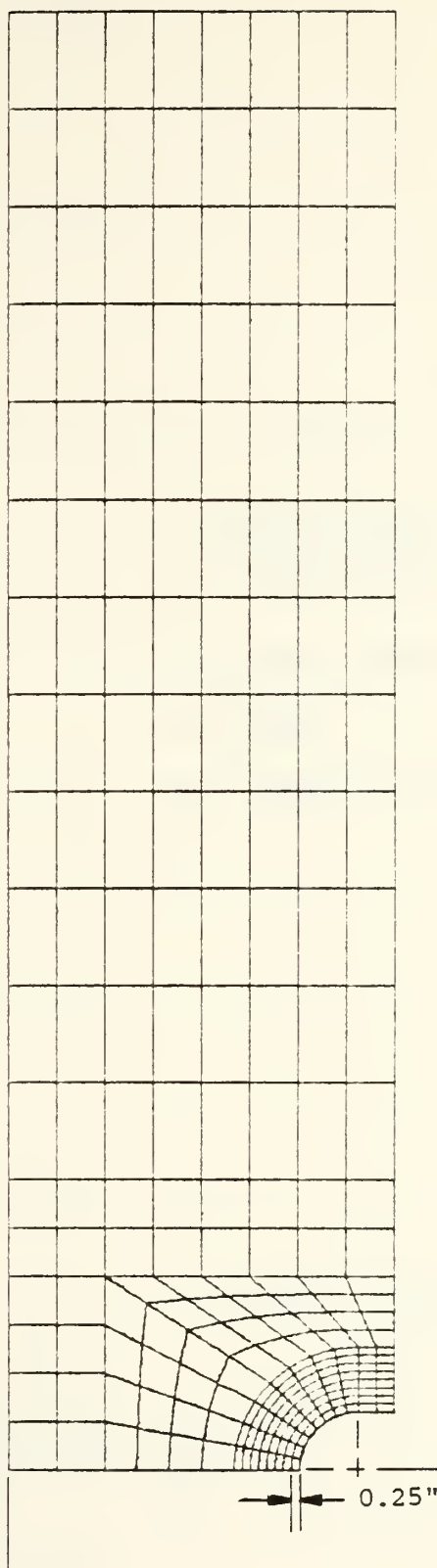
SUBDIVISION OF  
COURSE MESH

240 ELEMENTS  
(ISOPARAMETRIC)

797 NODES

1528 DEGREES OF  
FREEDOM

SYMMETRY  
AXES





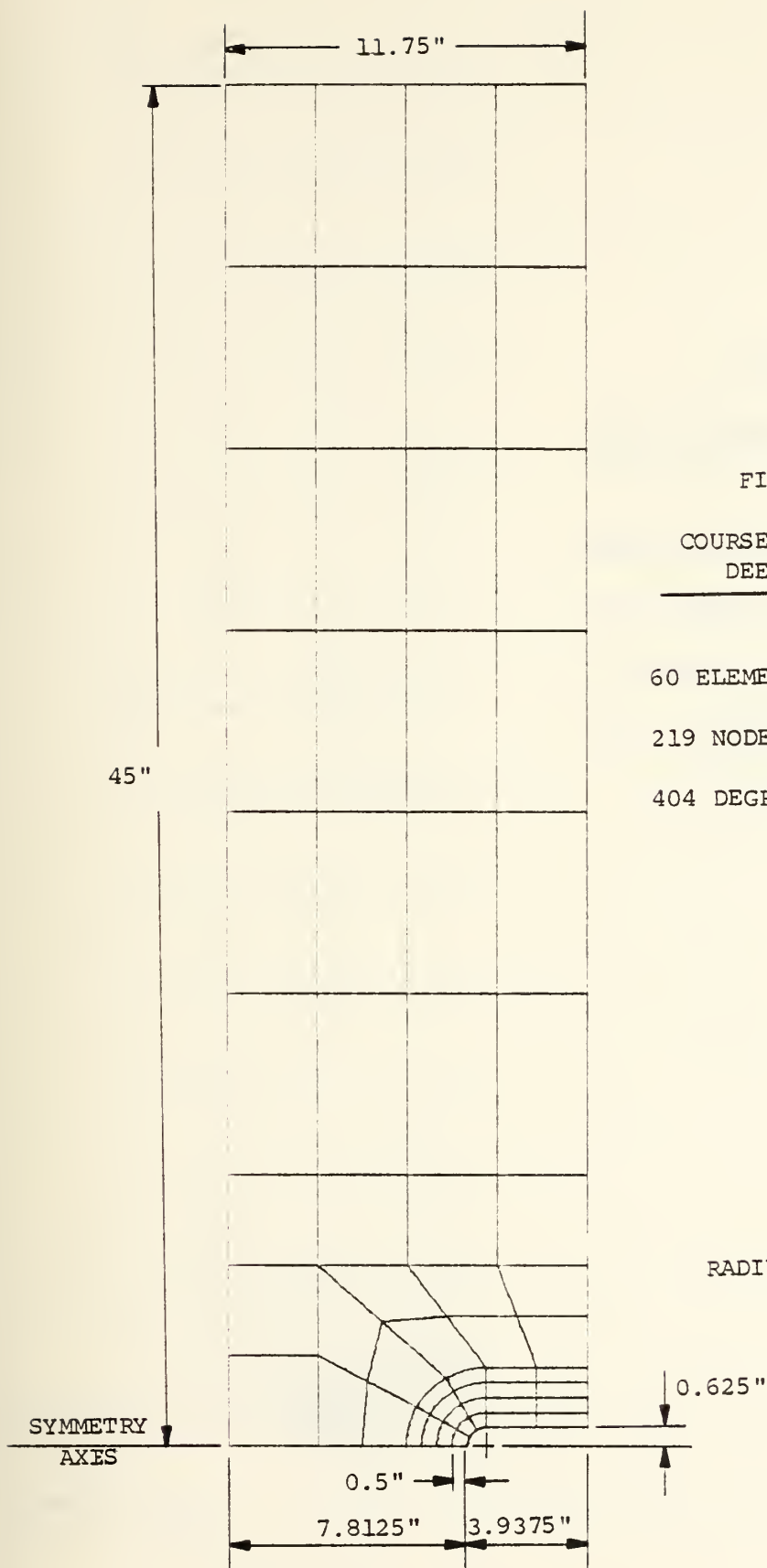


FIGURE 11

COURSE MESH FOR  
DEEP NOTCH

60 ELEMENTS (ISOPARAMETRIC)

219 NODES

404 DEGREES OF FREEDOM

RADIUS OF NOTCH= 0.625"



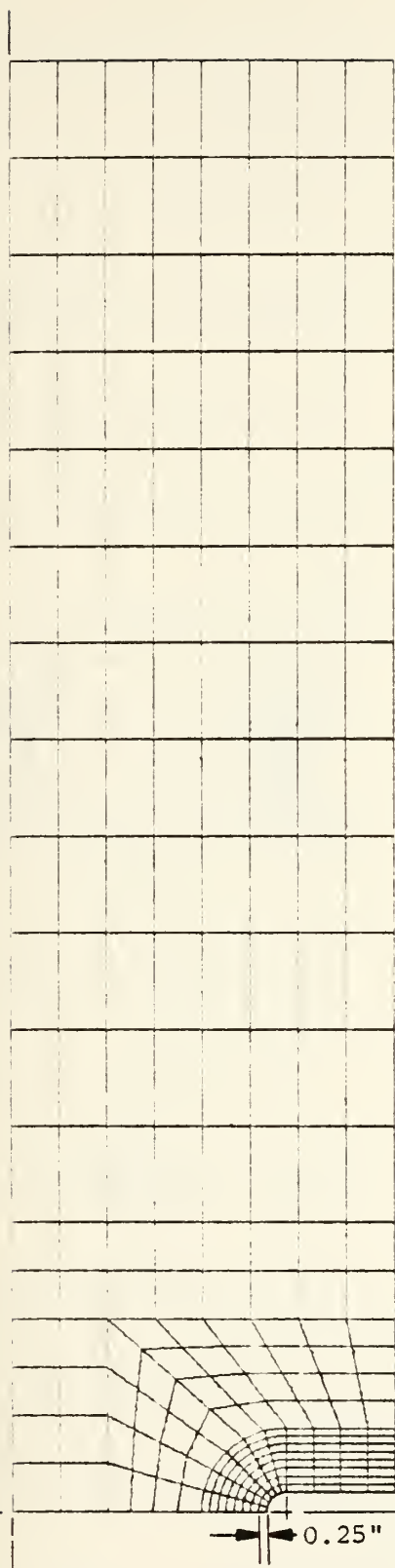


FIGURE 12

FINE MESH FOR DEEP NOTCH

SUBDIVISION OF COURSE MESH

240 ELEMENTS (ISOPARAMETRIC)

797 NODES

1528 DEGREES OF FREEDOM





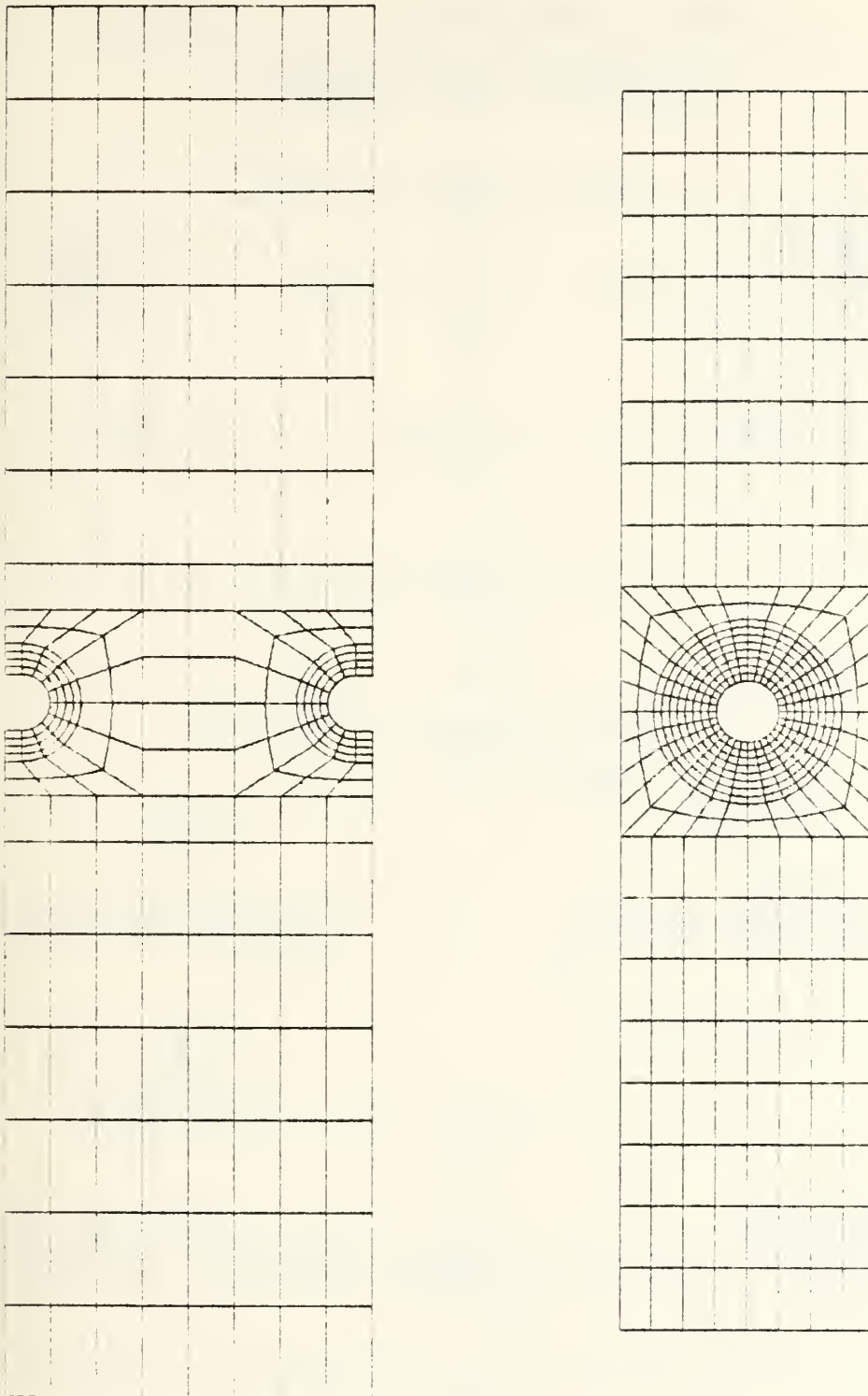


FIGURE 13

EXAMPLE OF COMPLETE PANEL MESHERS



FIGURE 14

COMPUTATIONAL FLOW CHART

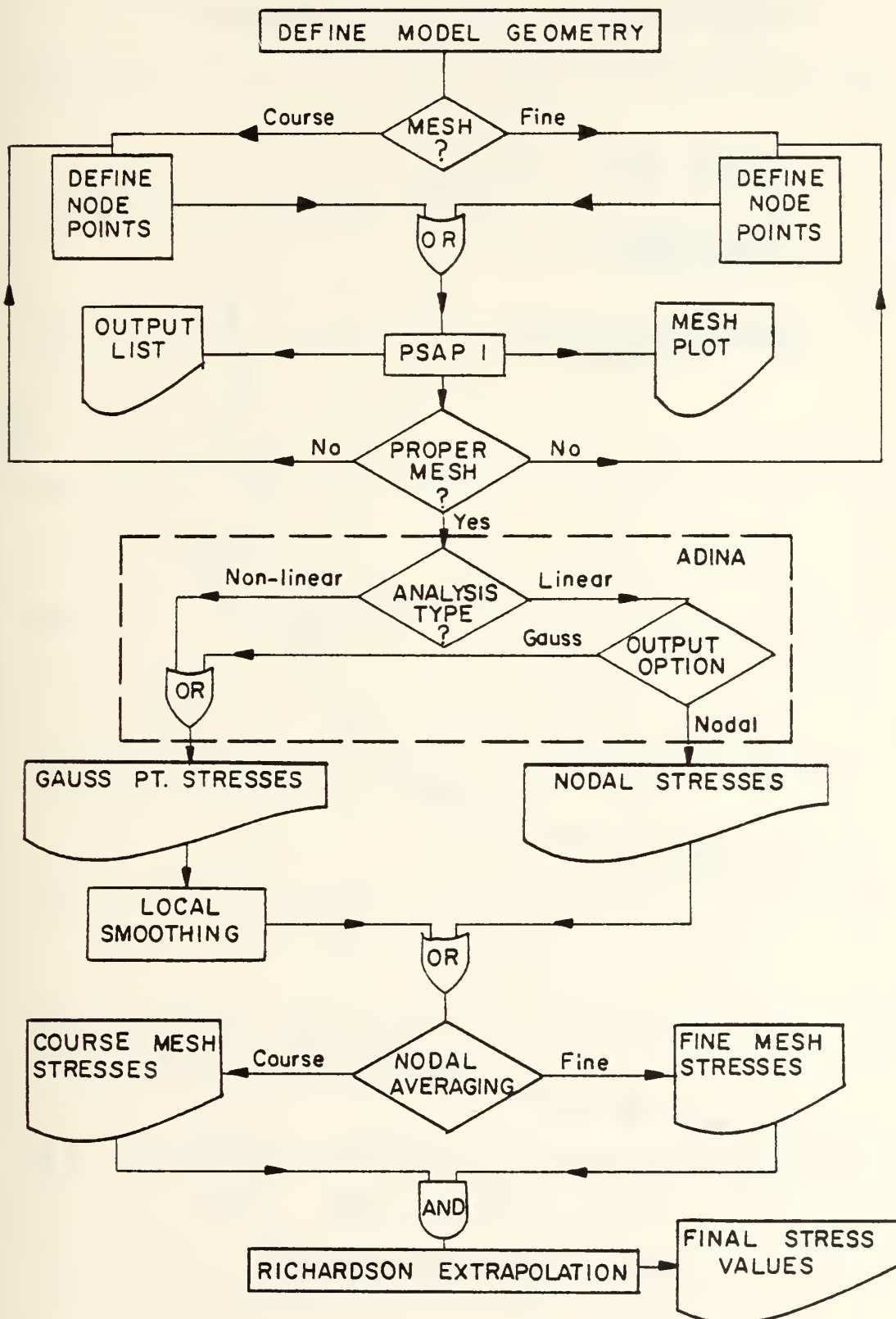




FIGURE 15  
CIRCULAR HOLE  $\lambda=0.2$  LINEAR RESULTS

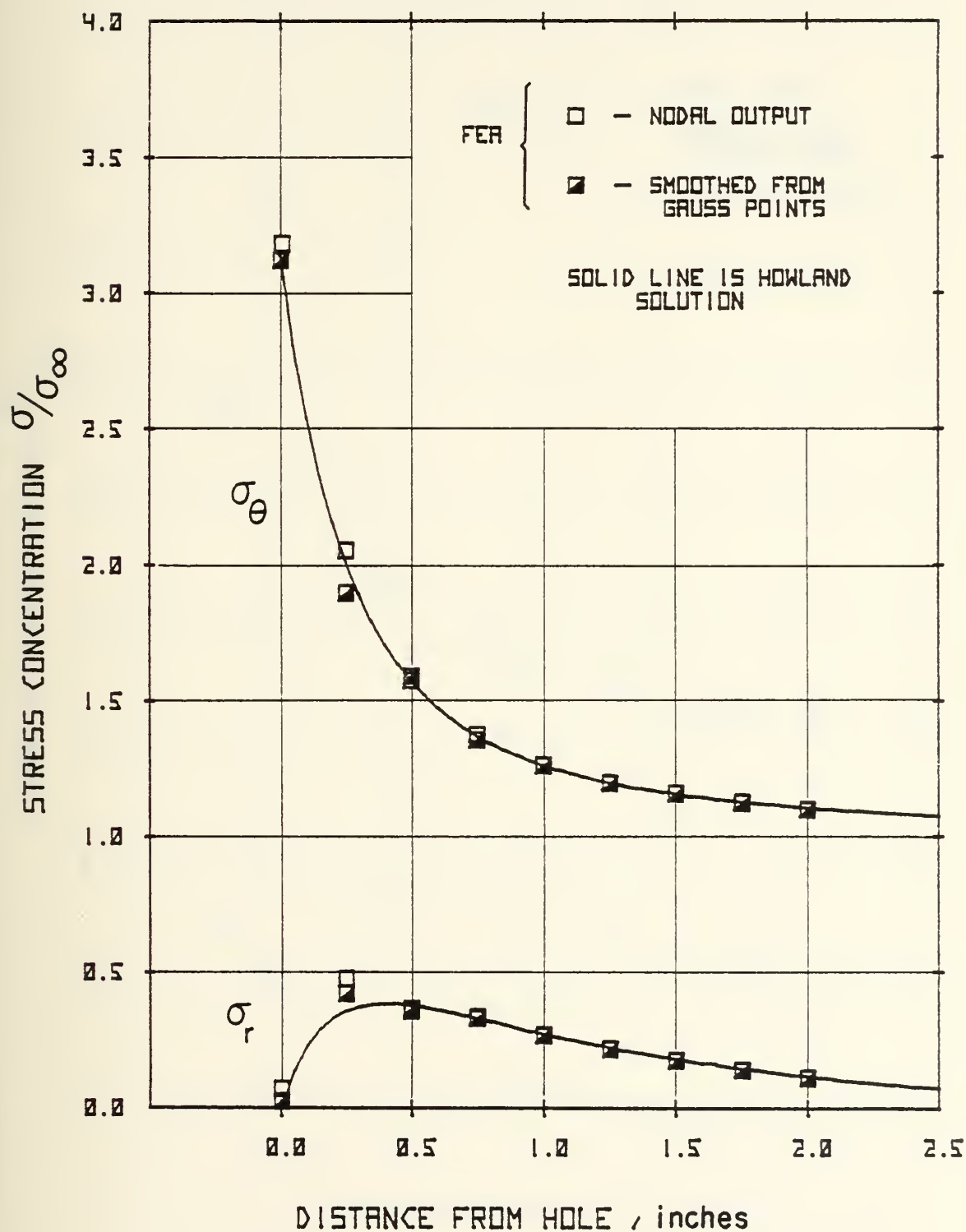




FIGURE 16

CIRCULAR HOLE  $\lambda=0.25$  LINEAR RESULTS

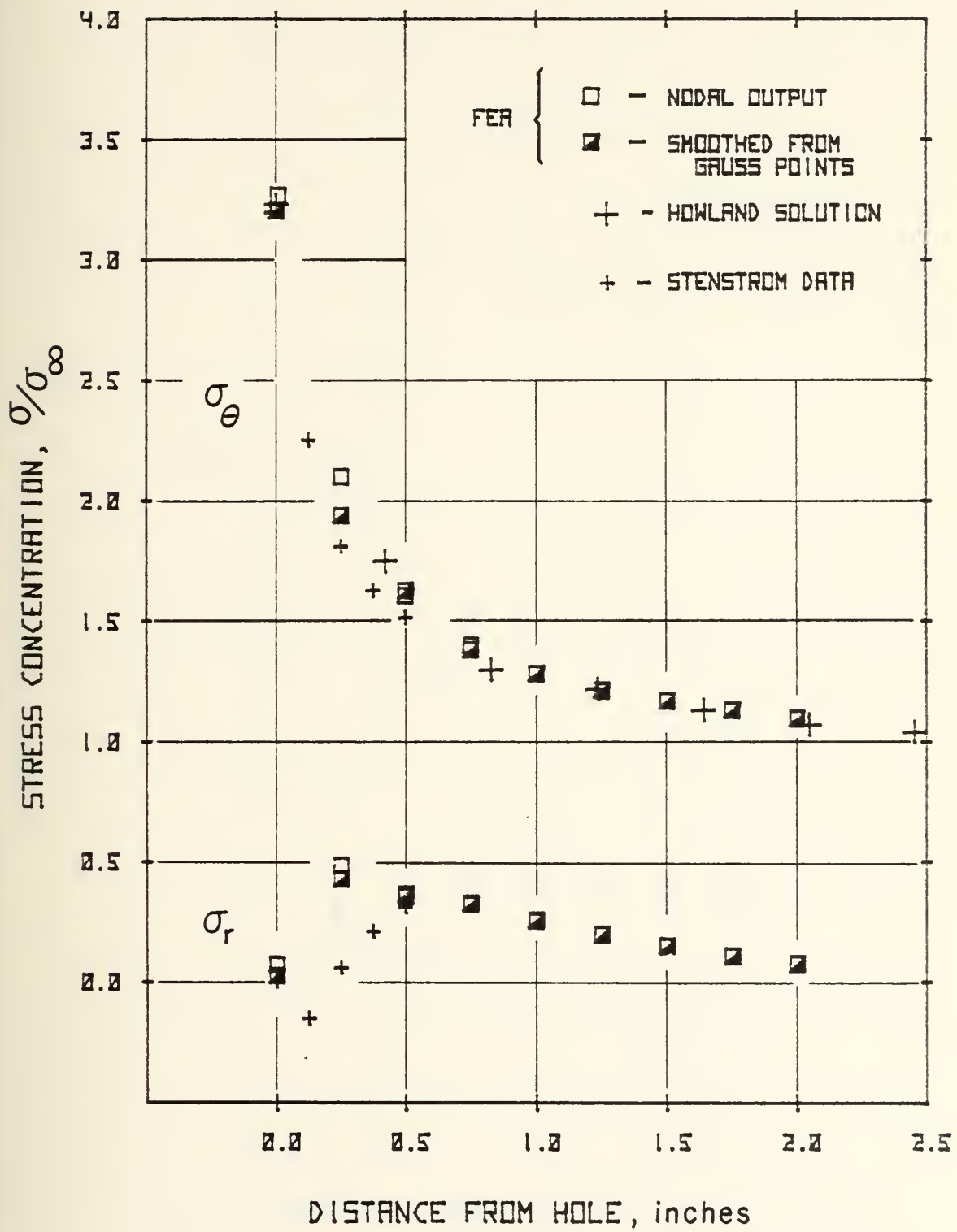






FIGURE 17

SHALLOW NOTCH LINEAR RESULTS

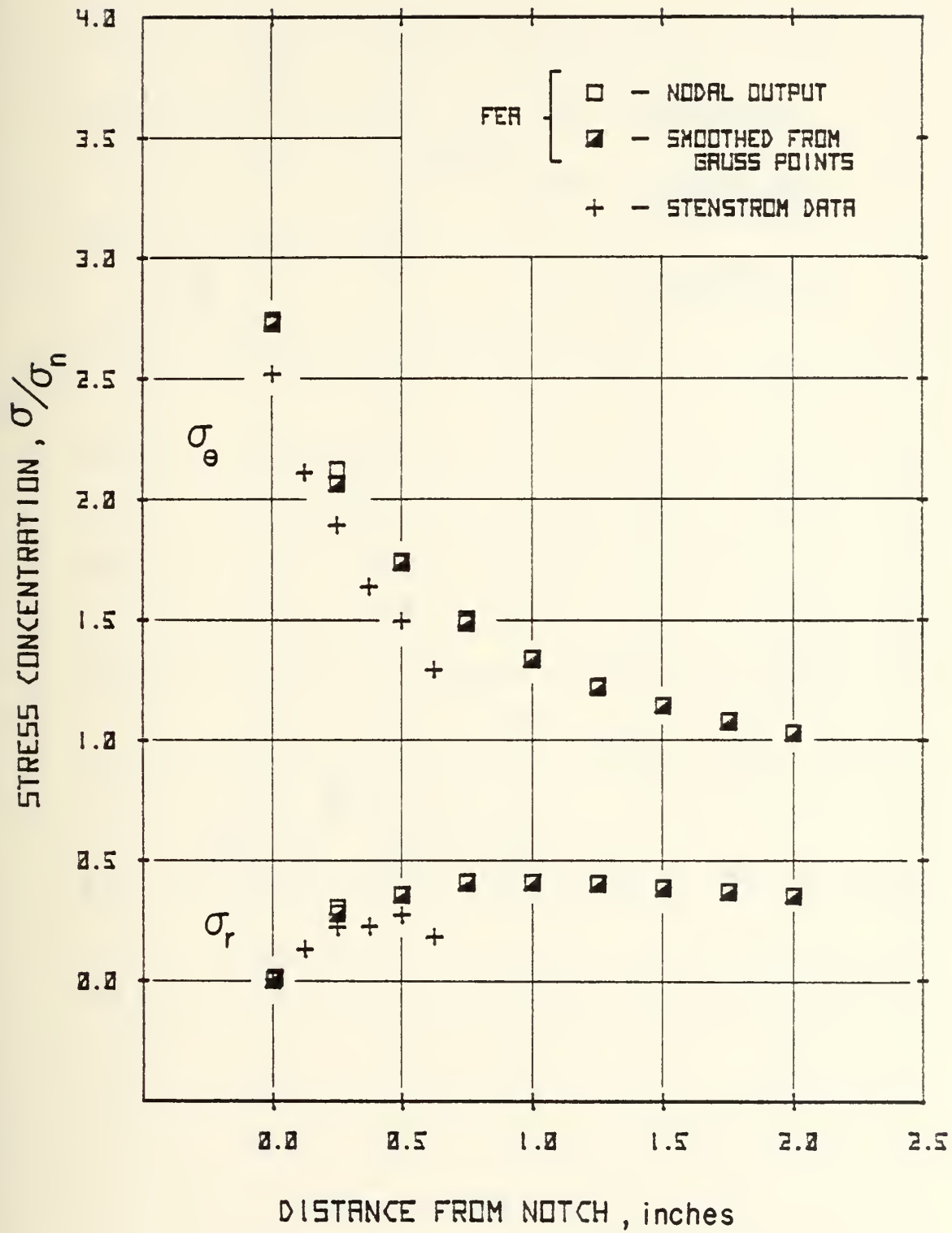




FIGURE 18  
DEEP NOTCH LINEAR RESULTS

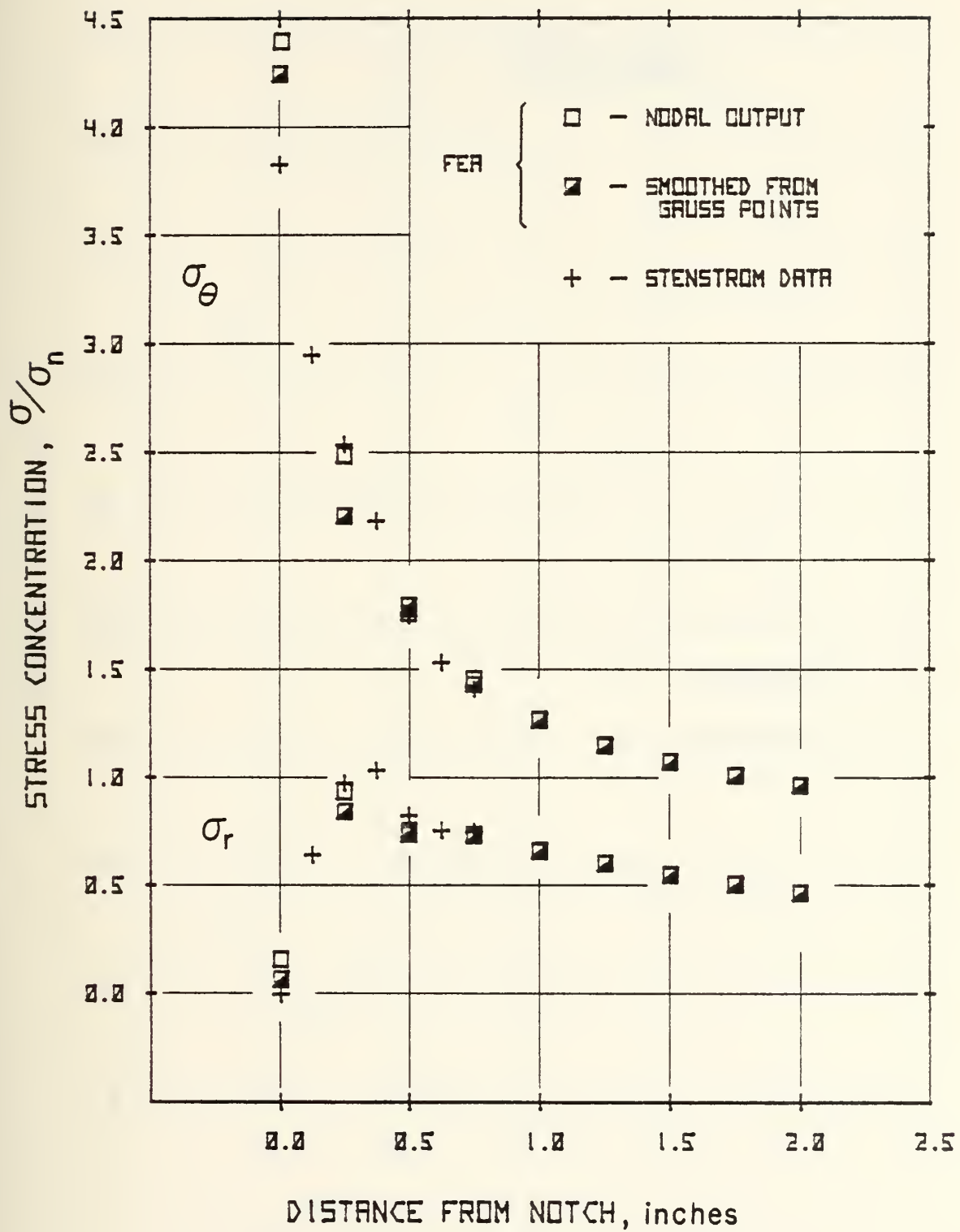




FIGURE 19

SHALLOW NOTCH 60000LB LOAD ELASTIC-PLASTIC RESULTS

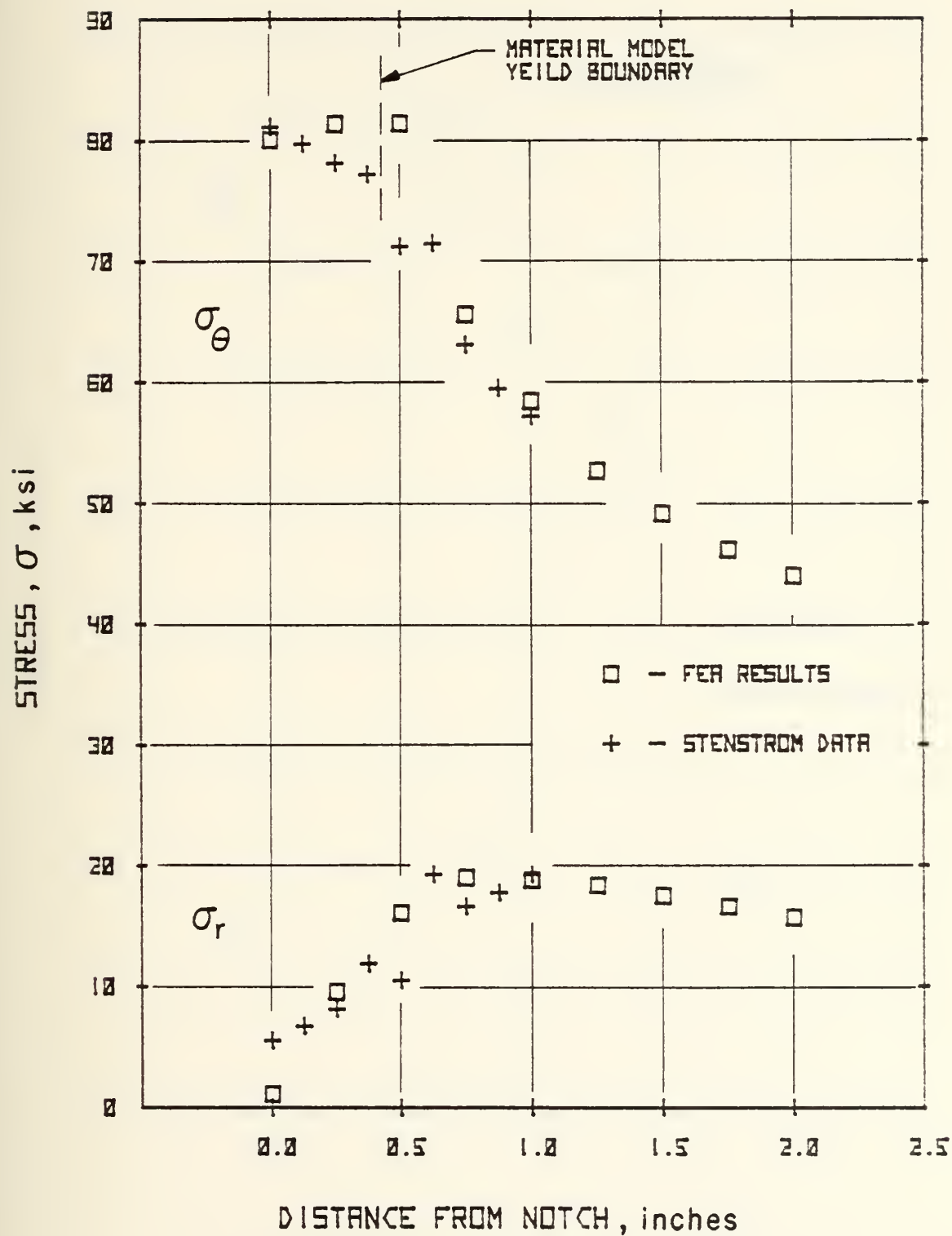




FIGURE 20

SHALLOW NOTCH 55000 LB LOAD ELASTIC-PLASTIC RESULTS

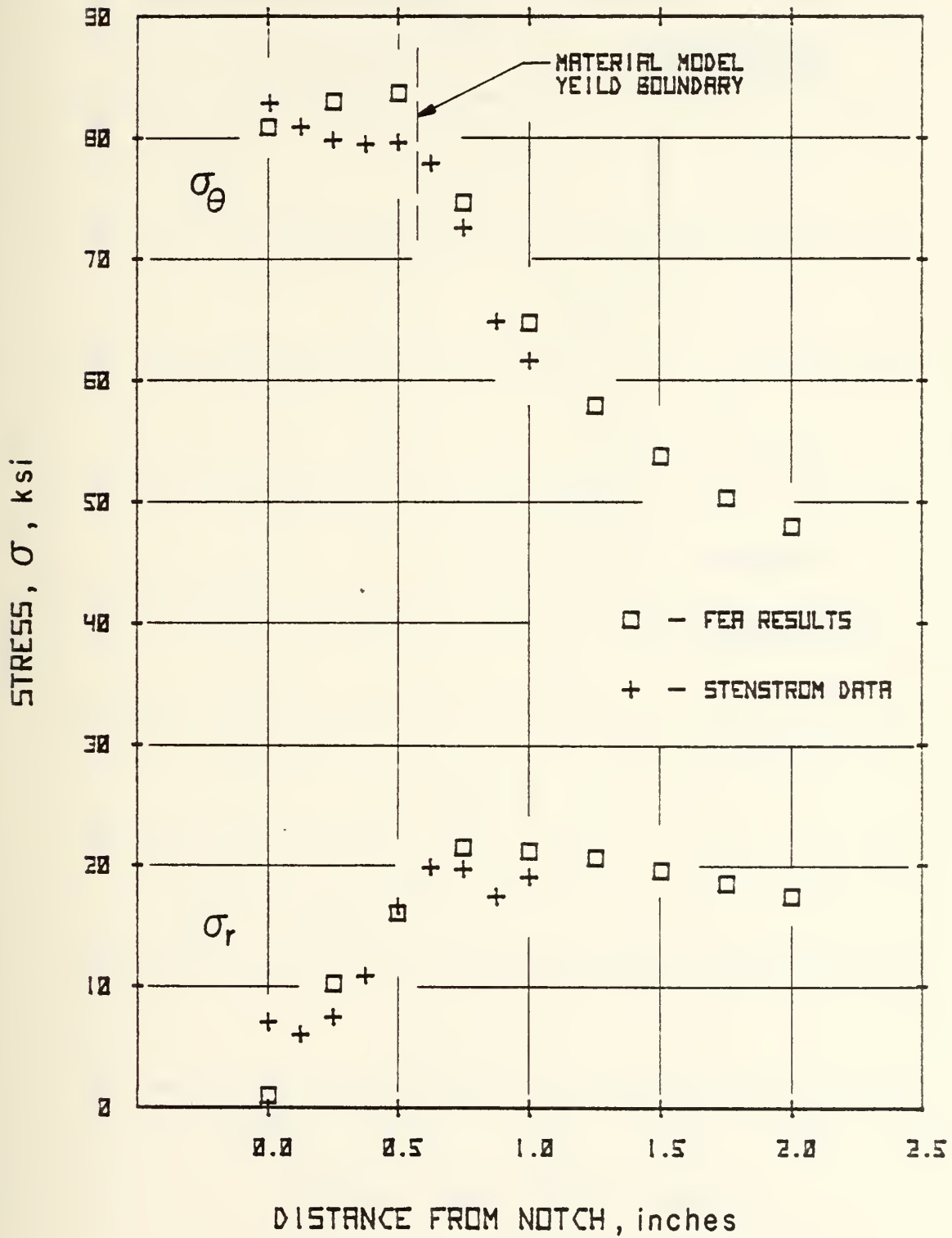
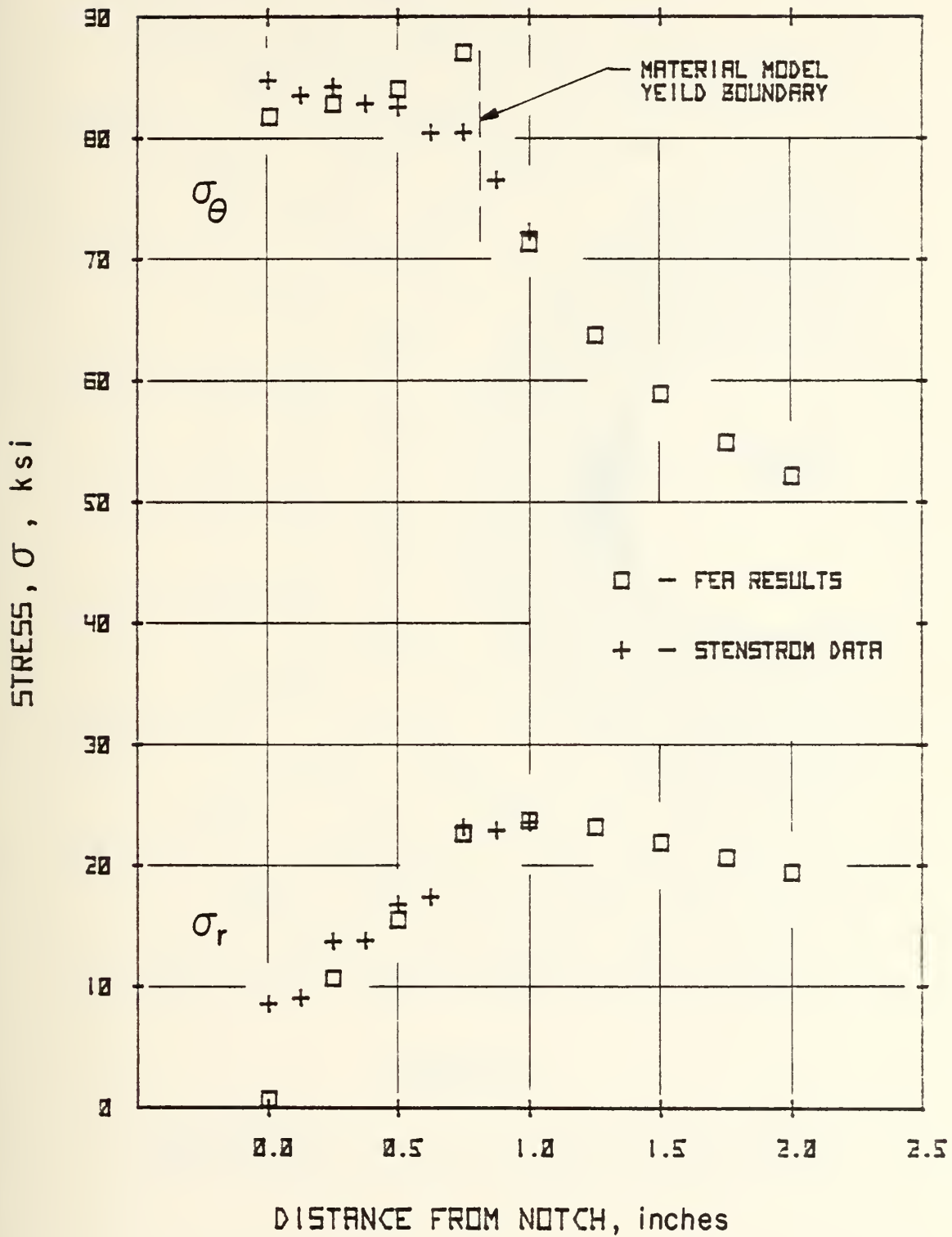






FIGURE 21

SHALLOW NOTCH 70000 LB LOAD ELASTIC-PLASTIC RESULTS





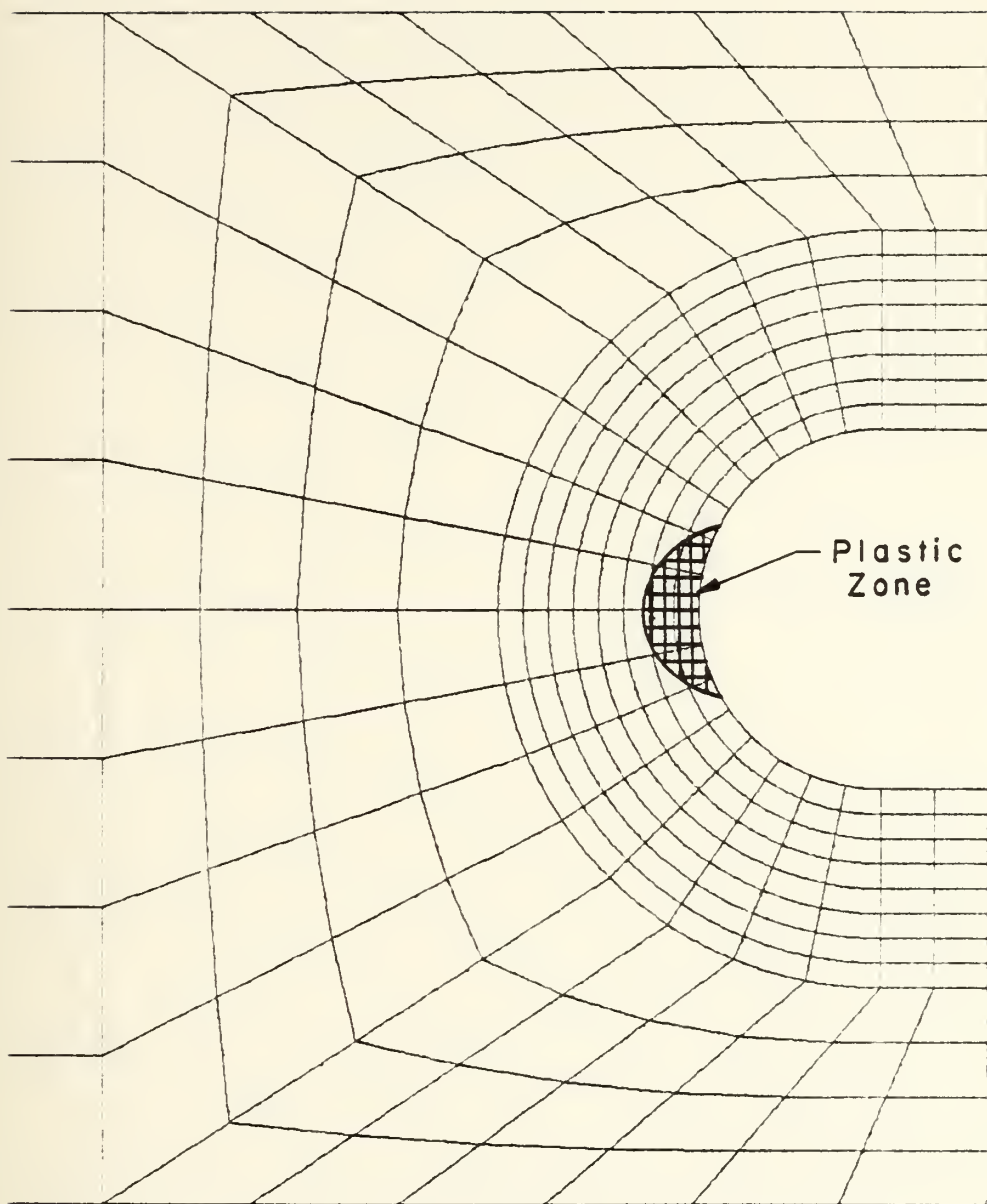


FIGURE . 22

SHALLOW NOTCH 60,000 LB LOAD PLASTIC ZONE



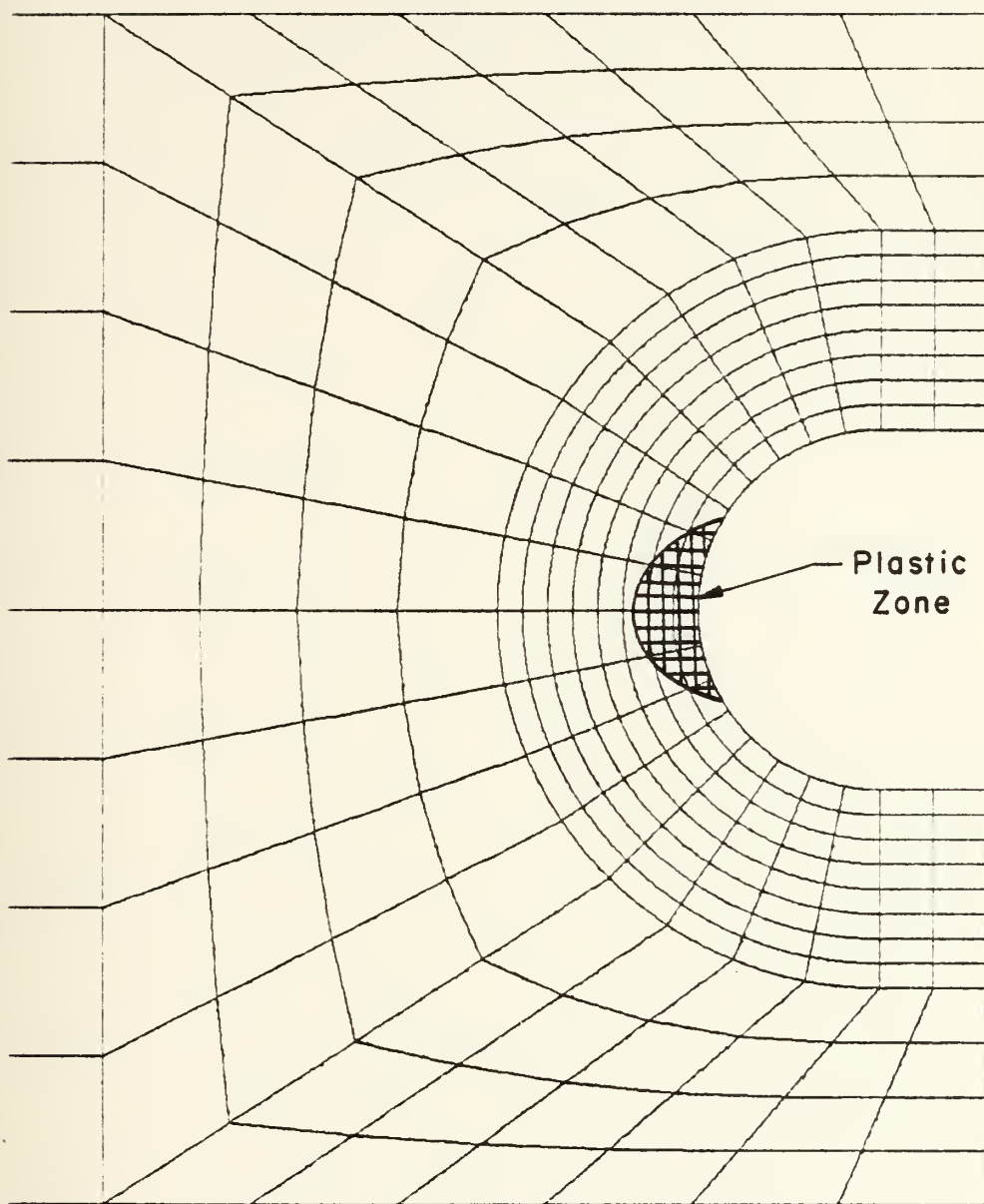


FIGURE 23

SHALLOW NOTCH 65,000 LB LOAD PLASTIC ZONE



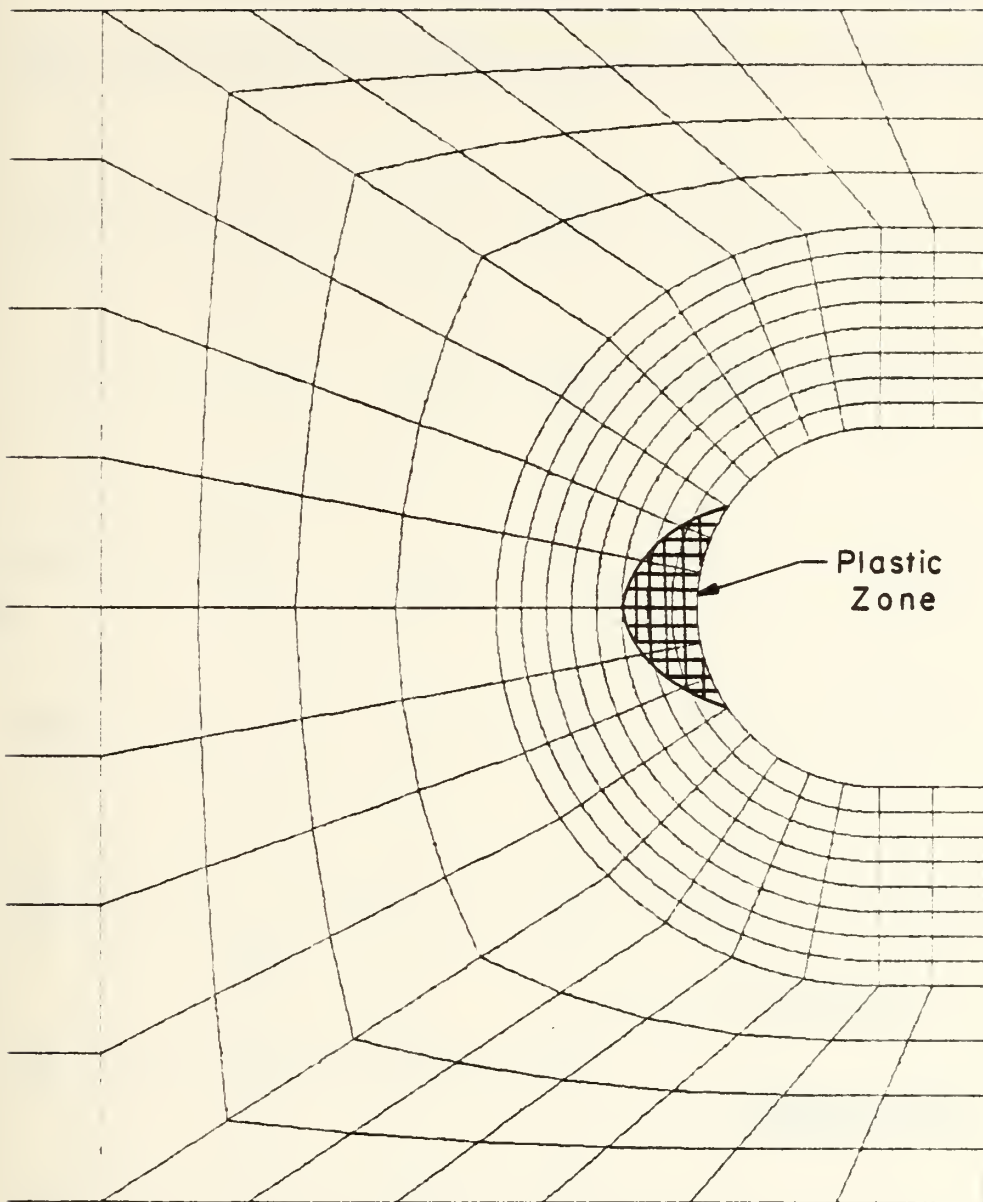


FIGURE 24

SHALLOW NOTCH 70,000 LB LOAD PLASTIC ZONE





FIGURE 25

SHALLOW NOTCH RESIDUAL  $\sigma_{\theta}$  FROM 60,000 LB LOAD

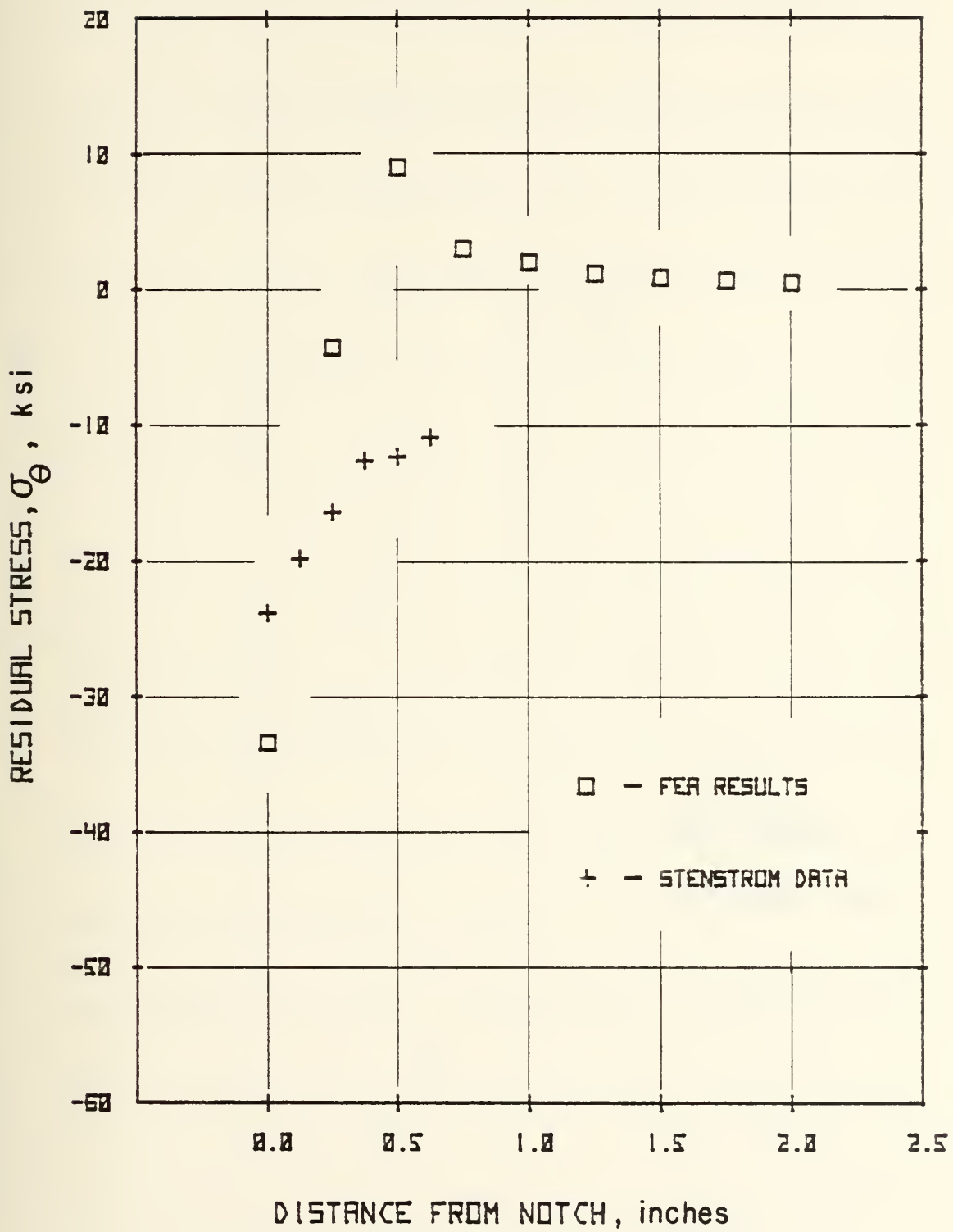




FIGURE 26

SHALLOW NOTCH RESIDUAL  $\sigma_r$  FROM 60,000 LB LOAD

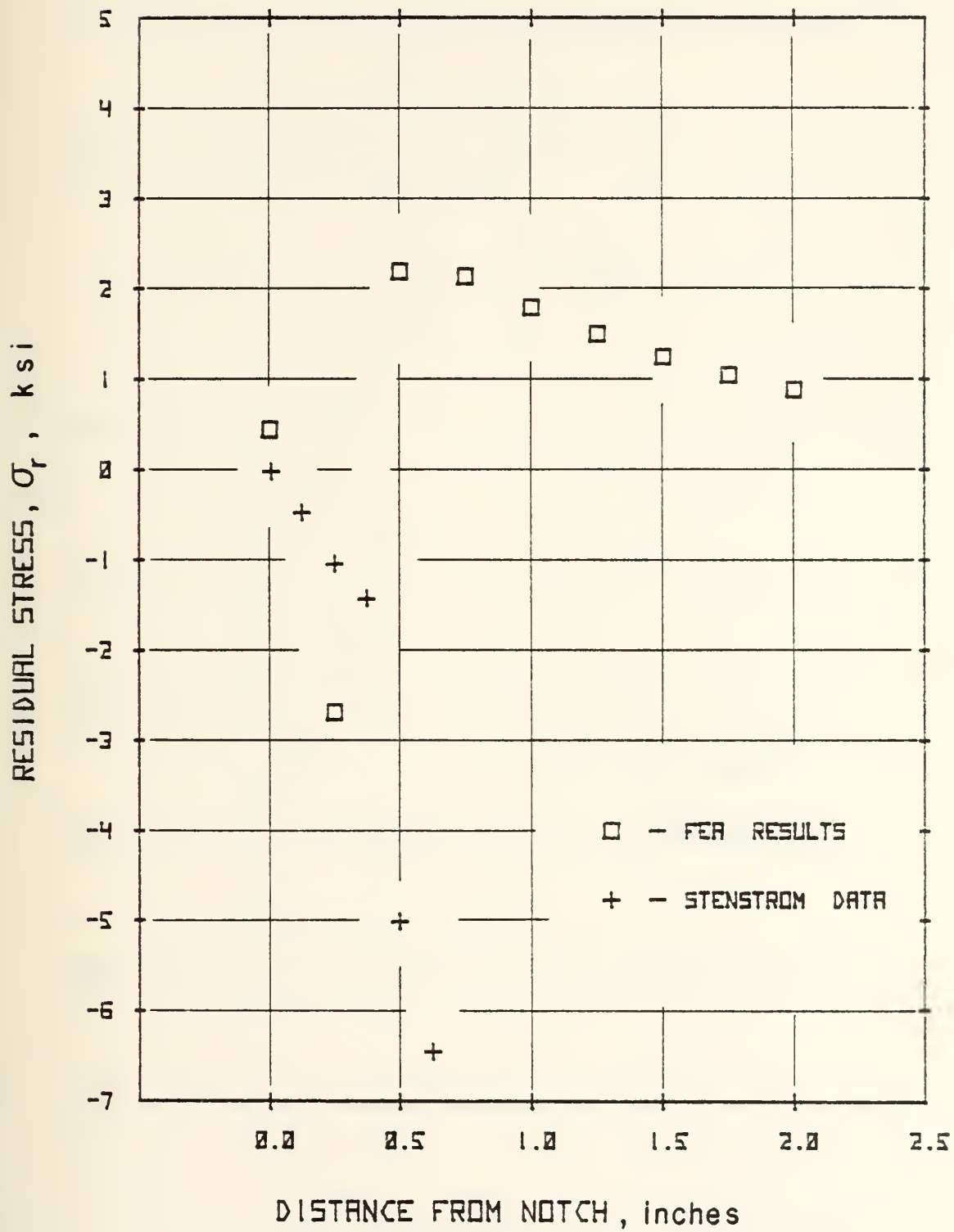




FIGURE 27

SHALLOW NOTCH RESIDUAL  $\sigma_{\theta}$  FROM 65,000 LB LOAD

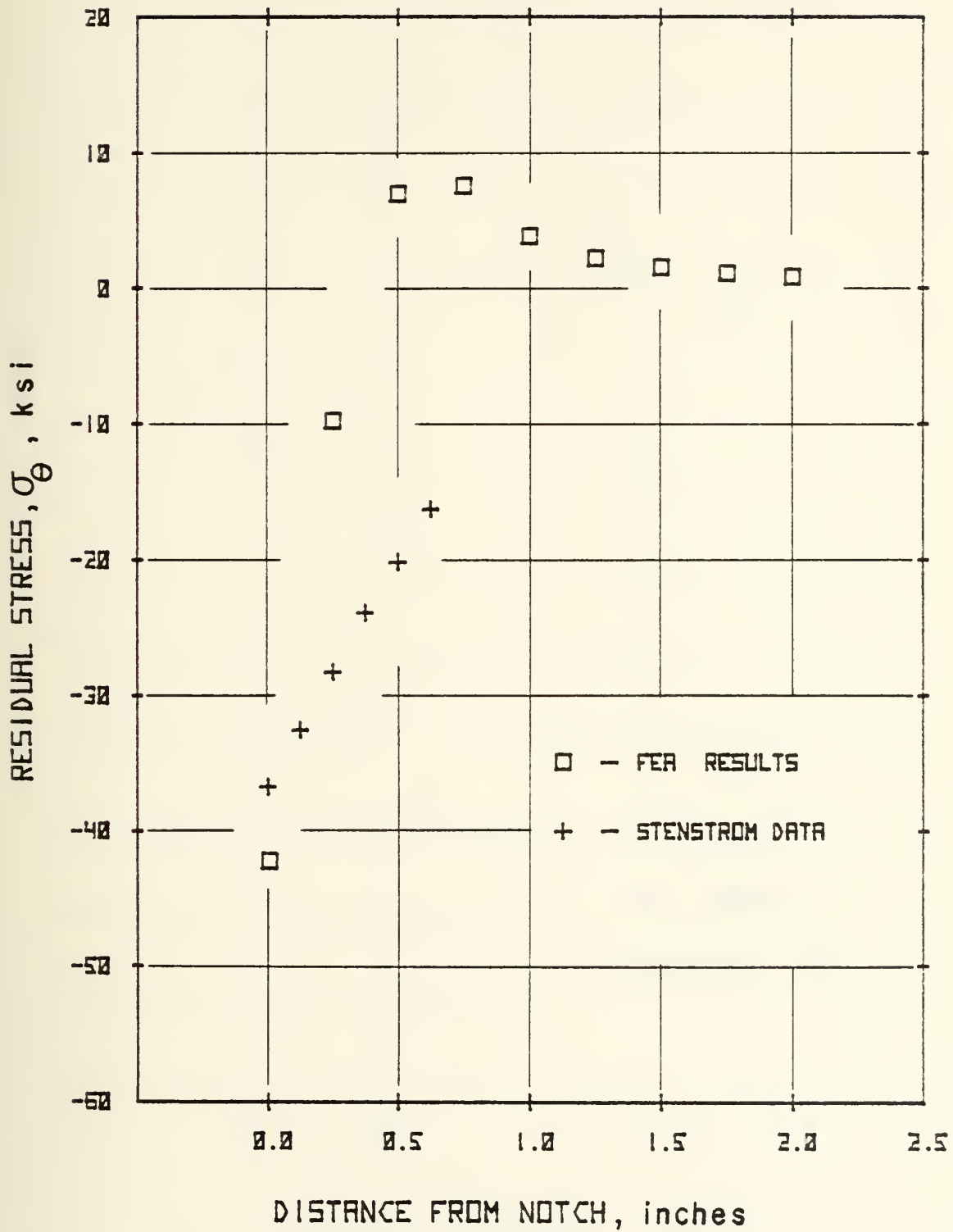




FIGURE 28

SHALLOW NOTCH RESIDUAL  $\sigma_r$  FROM 55,000 LB LOAD

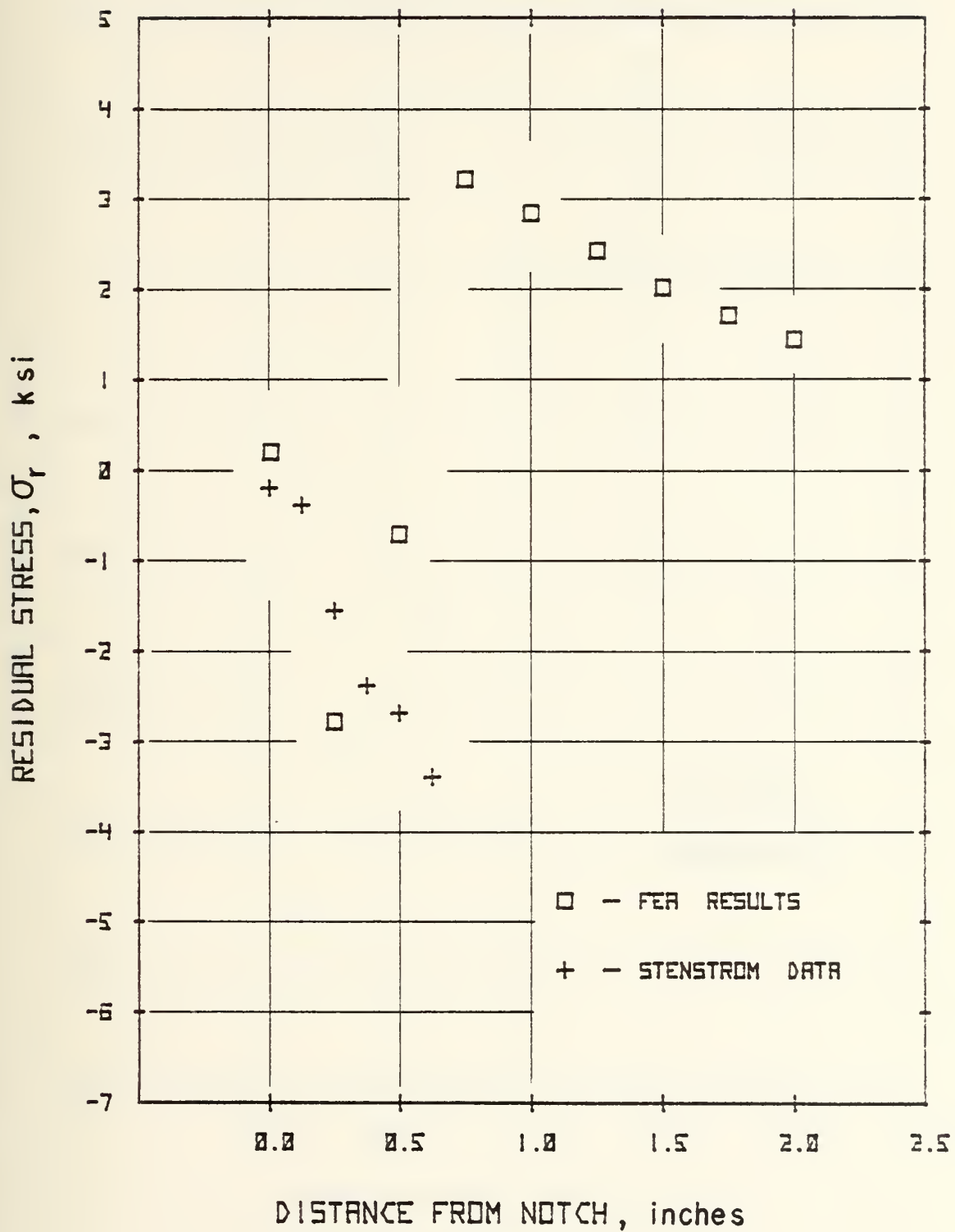






FIGURE 29

SHALLOW NOTCH RESIDUAL  $\sigma_{\theta}$  FROM 70,000 LB LOAD

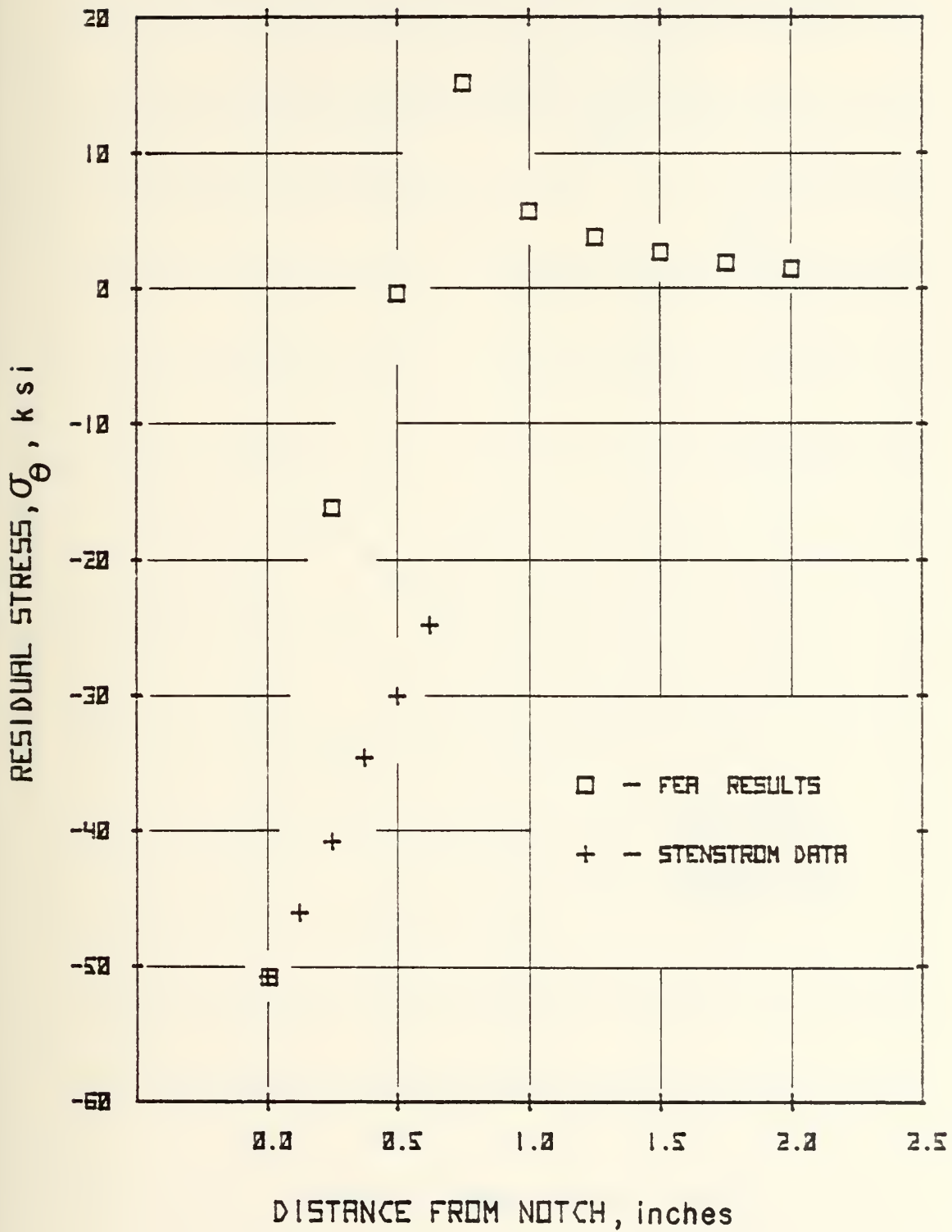




FIGURE 30

SHALLOW NOTCH RESIDUAL  $\sigma_r$  FROM 70,000 LB LOAD

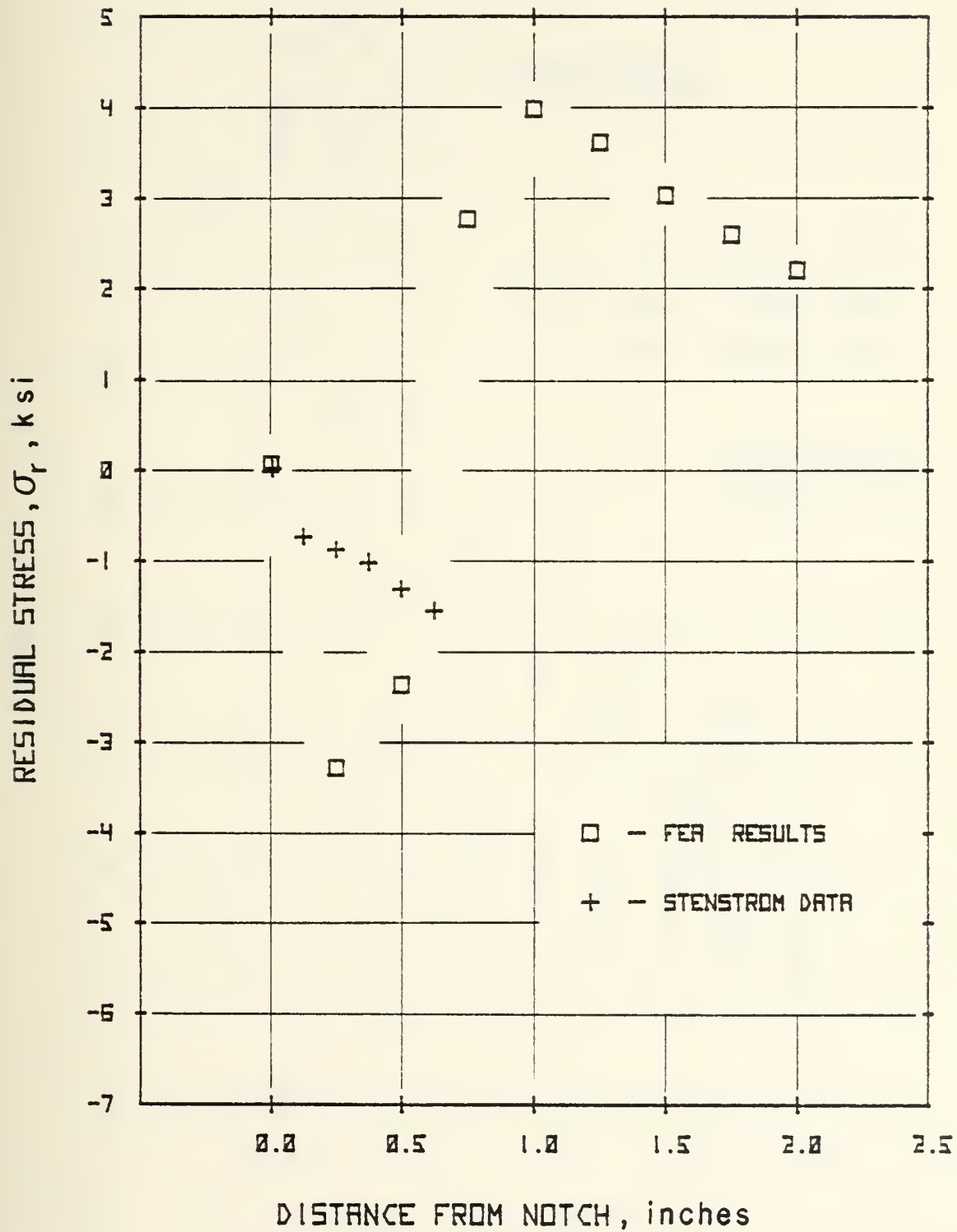




FIGURE 31

DEEP NOTCH PLASTIC LOADING RESULTS

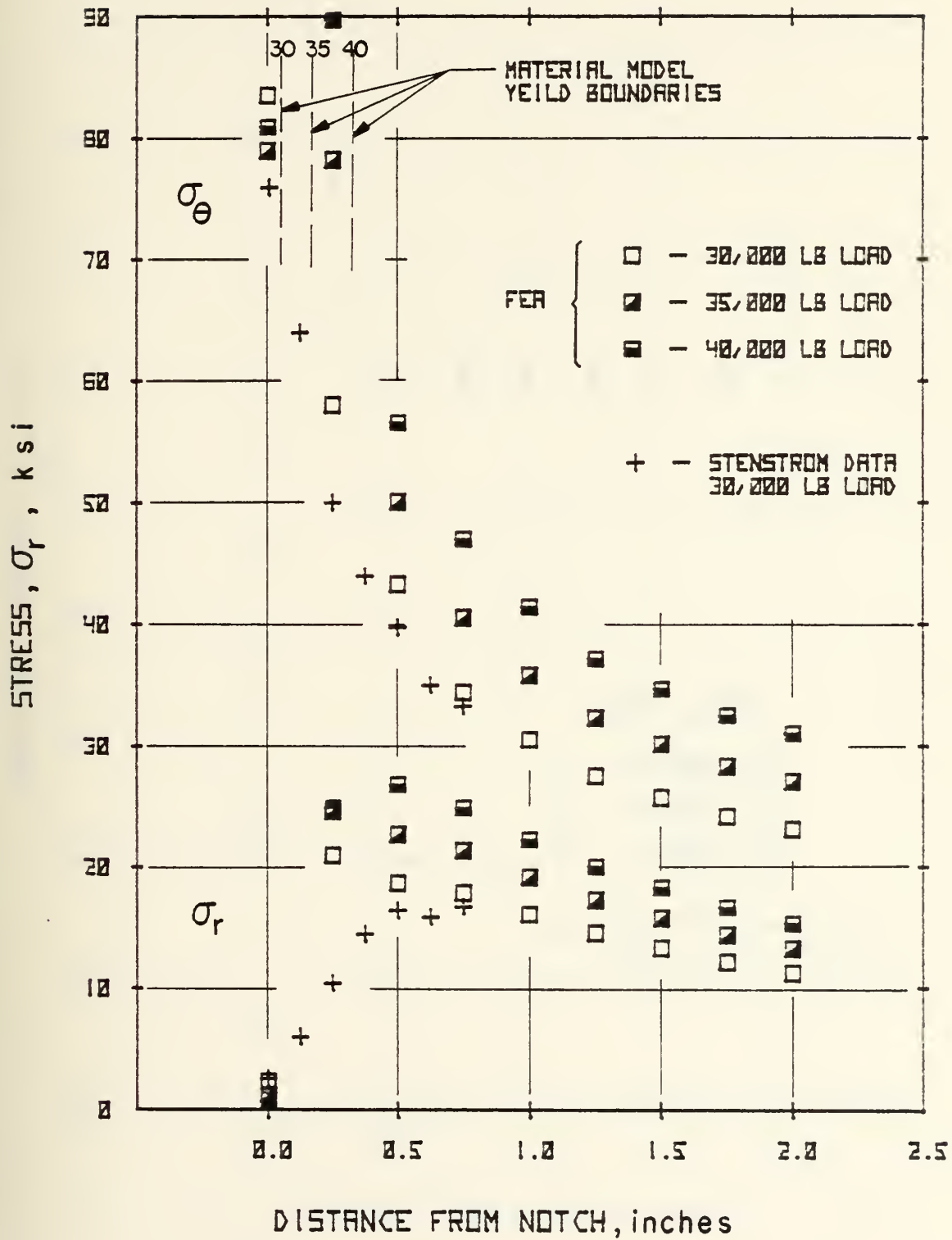




FIGURE 32

DEEP NOTCH  $\sigma_\theta$  RESIDUALS

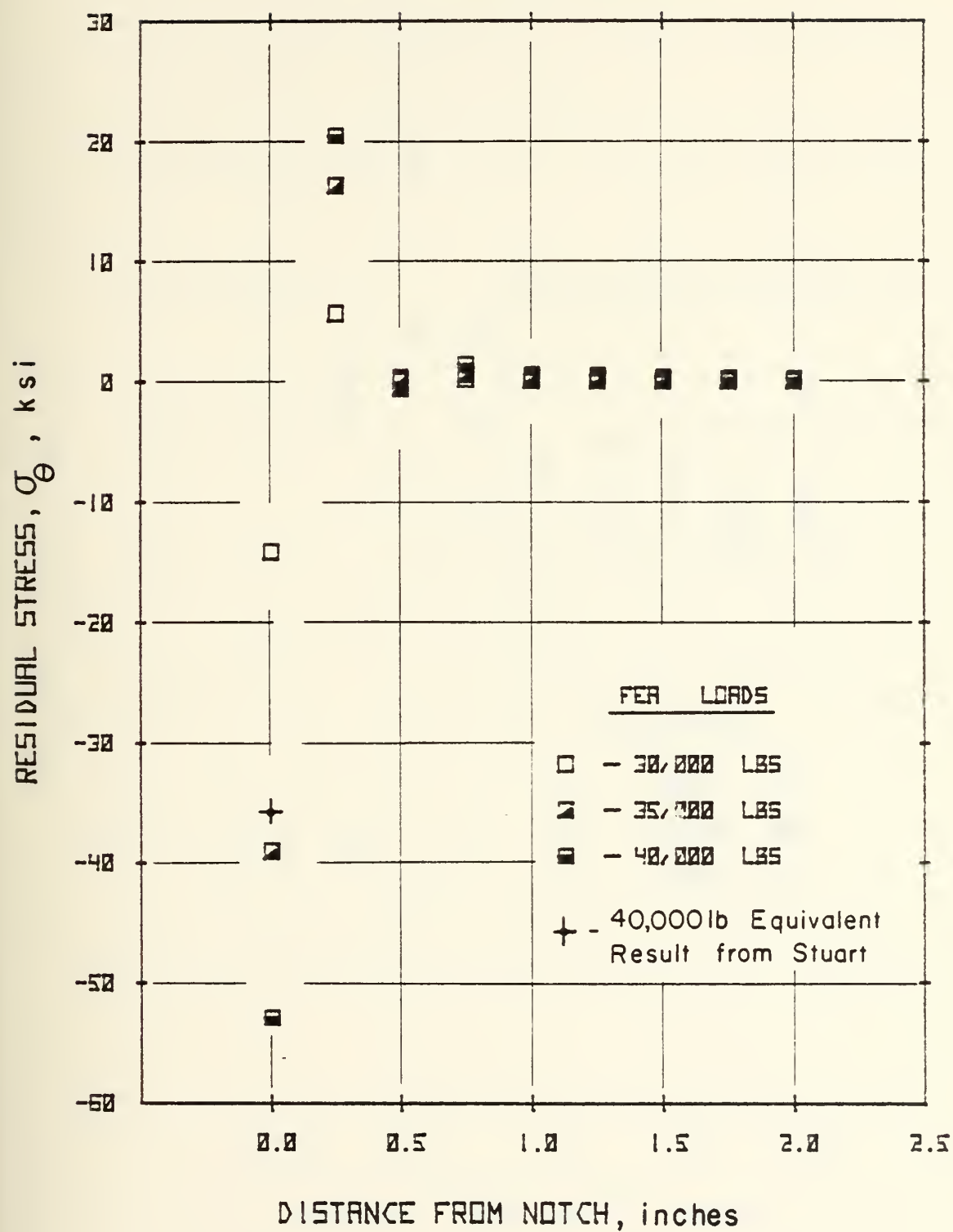
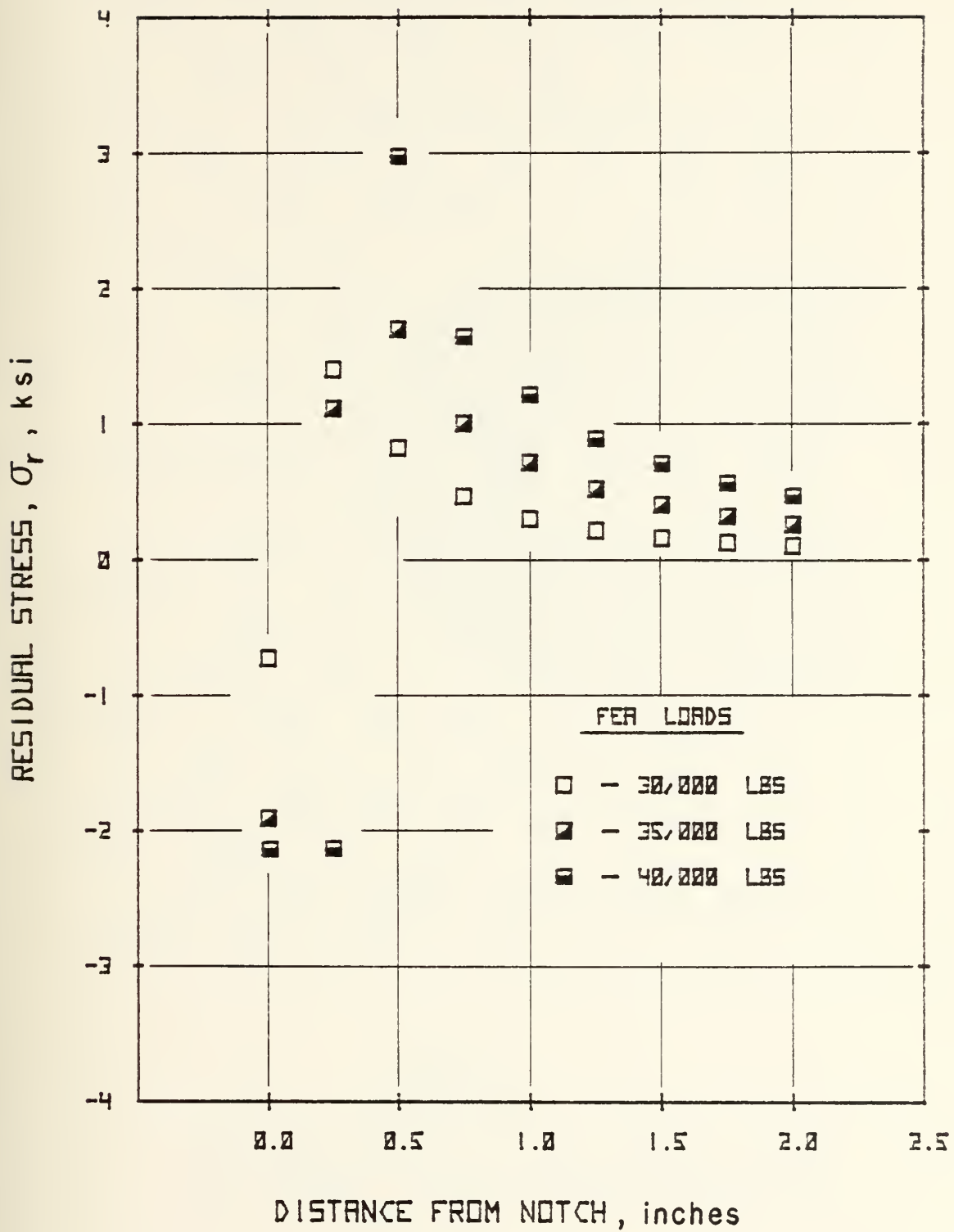






FIGURE 33

DEEP NOTCH  $\sigma_r$  RESIDUALS





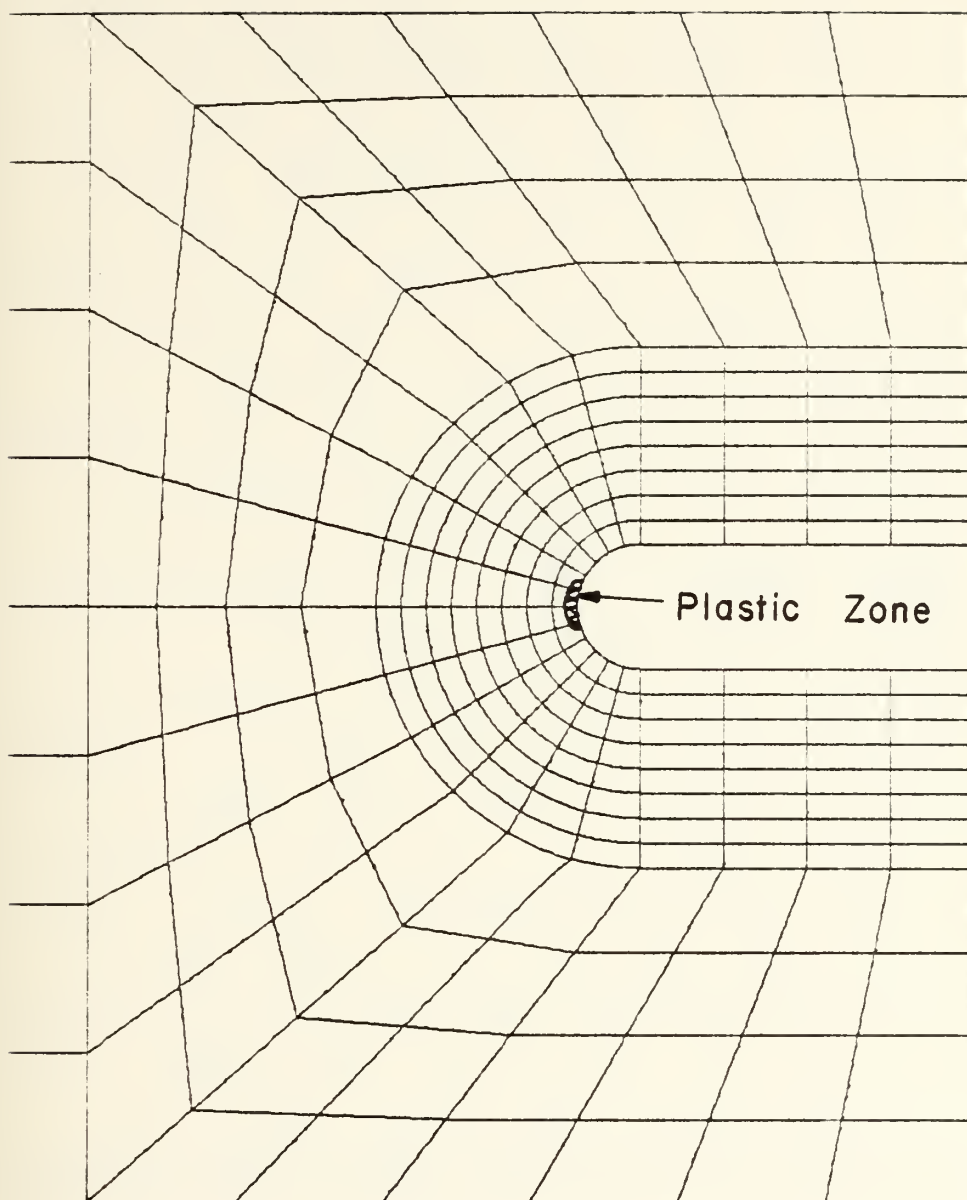


FIGURE 34

DEEP NOTCH 30,000 LB LOAD PLASTIC ZONE



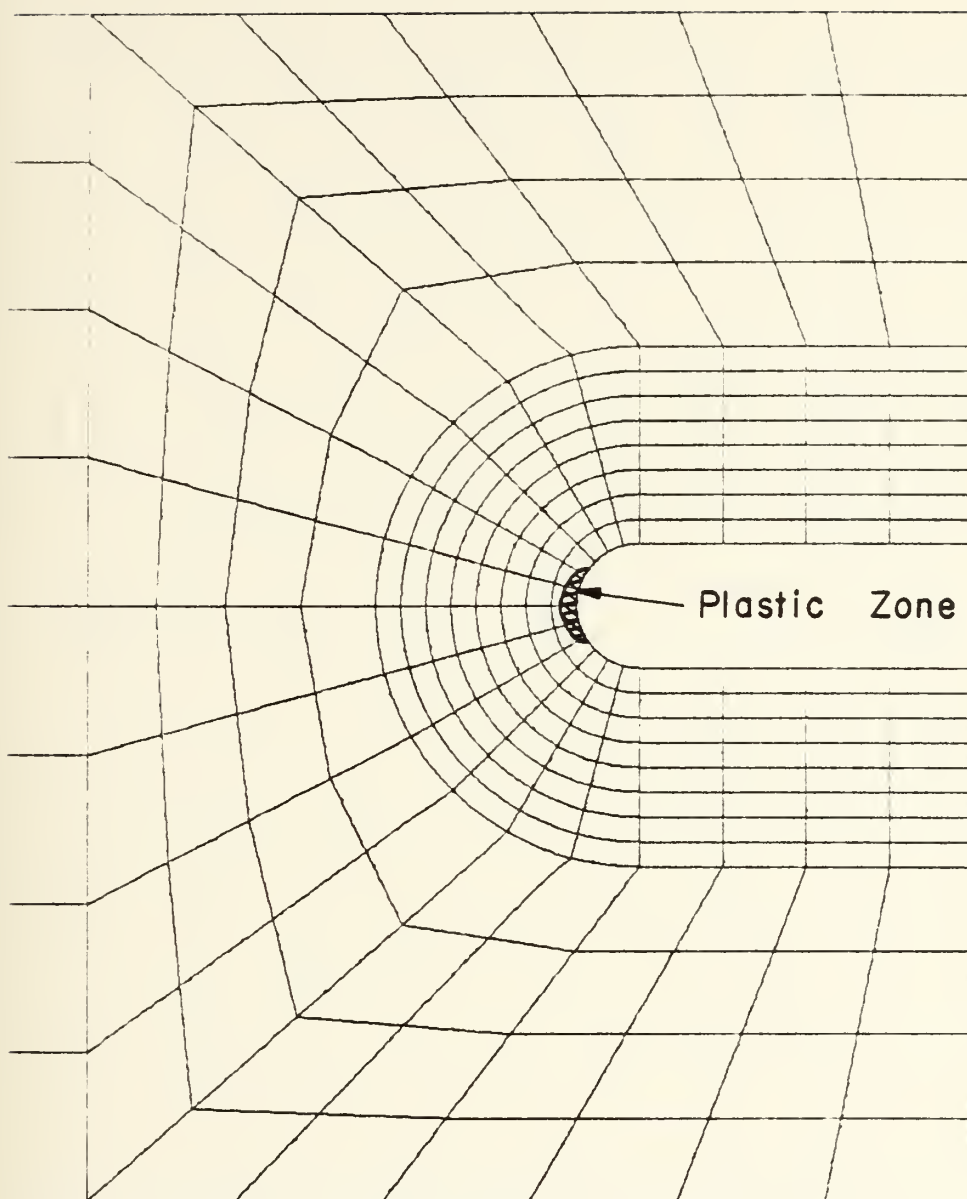


FIGURE 35

DEEP NOTCH 35,000 LB LOAD PLASTIC ZONE



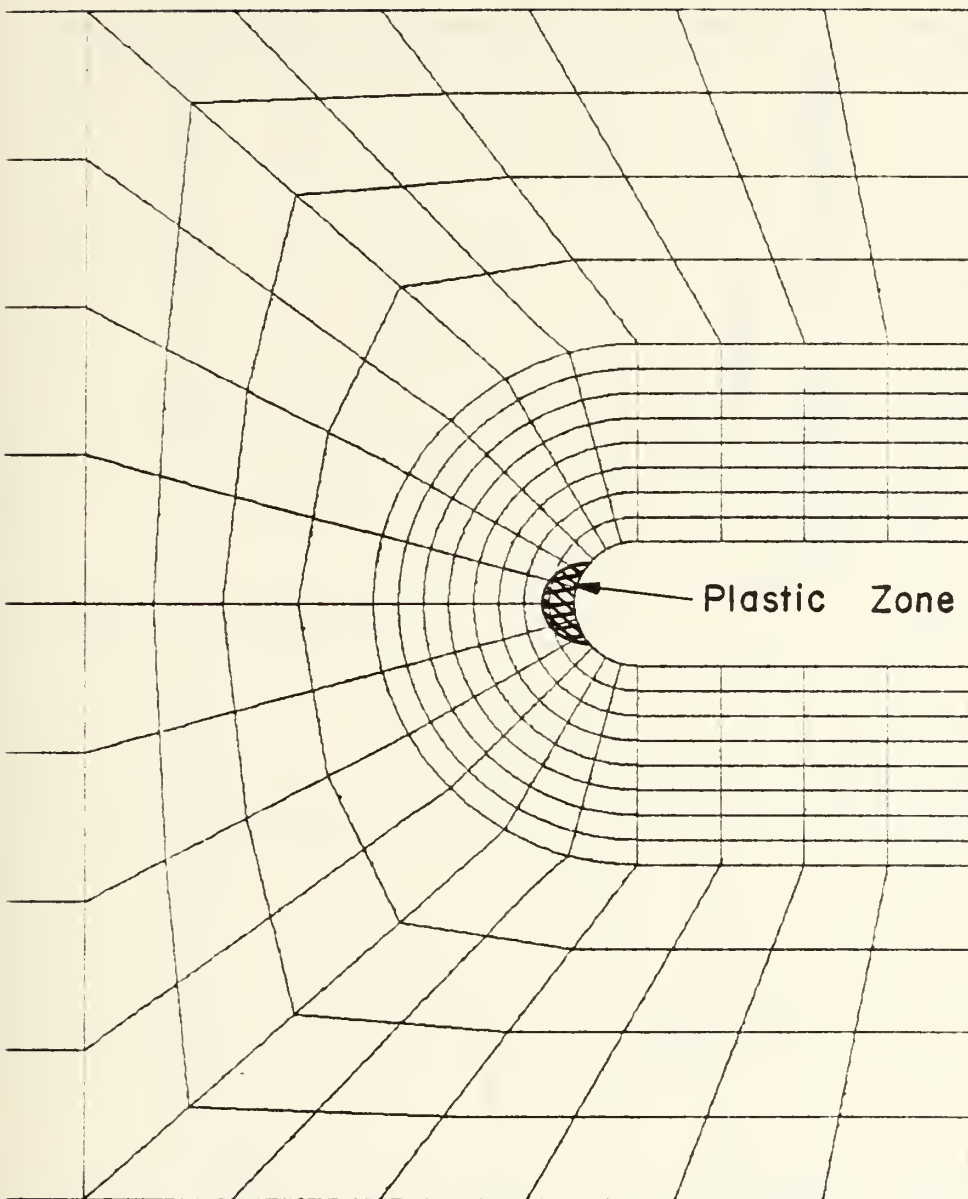


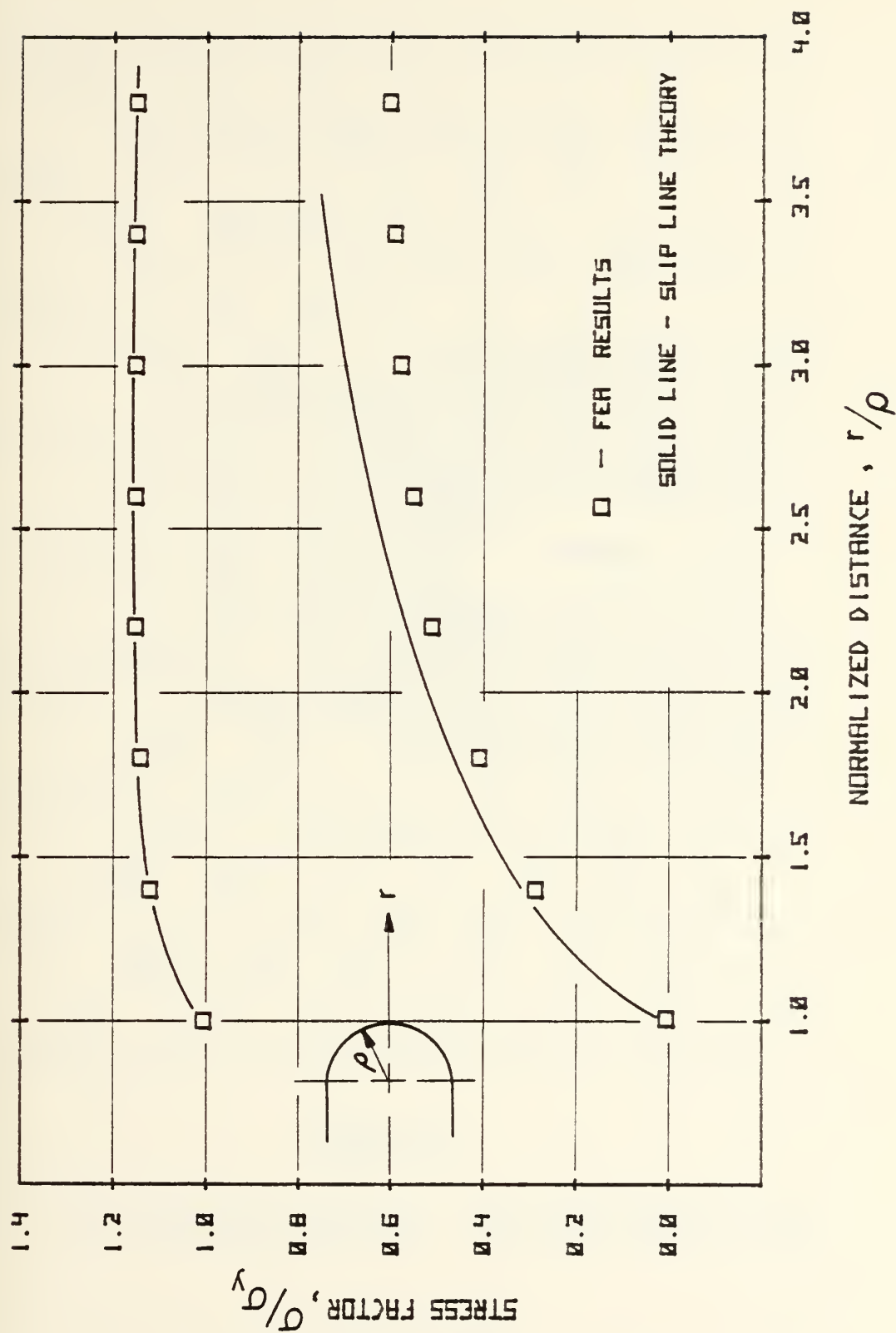
FIGURE 36

DEEP NOTCH 40,000 LB LOAD PLASTIC ZONE





FIGURE 37  
RIGID-PERFECTLY-PLASTIC RESULTS





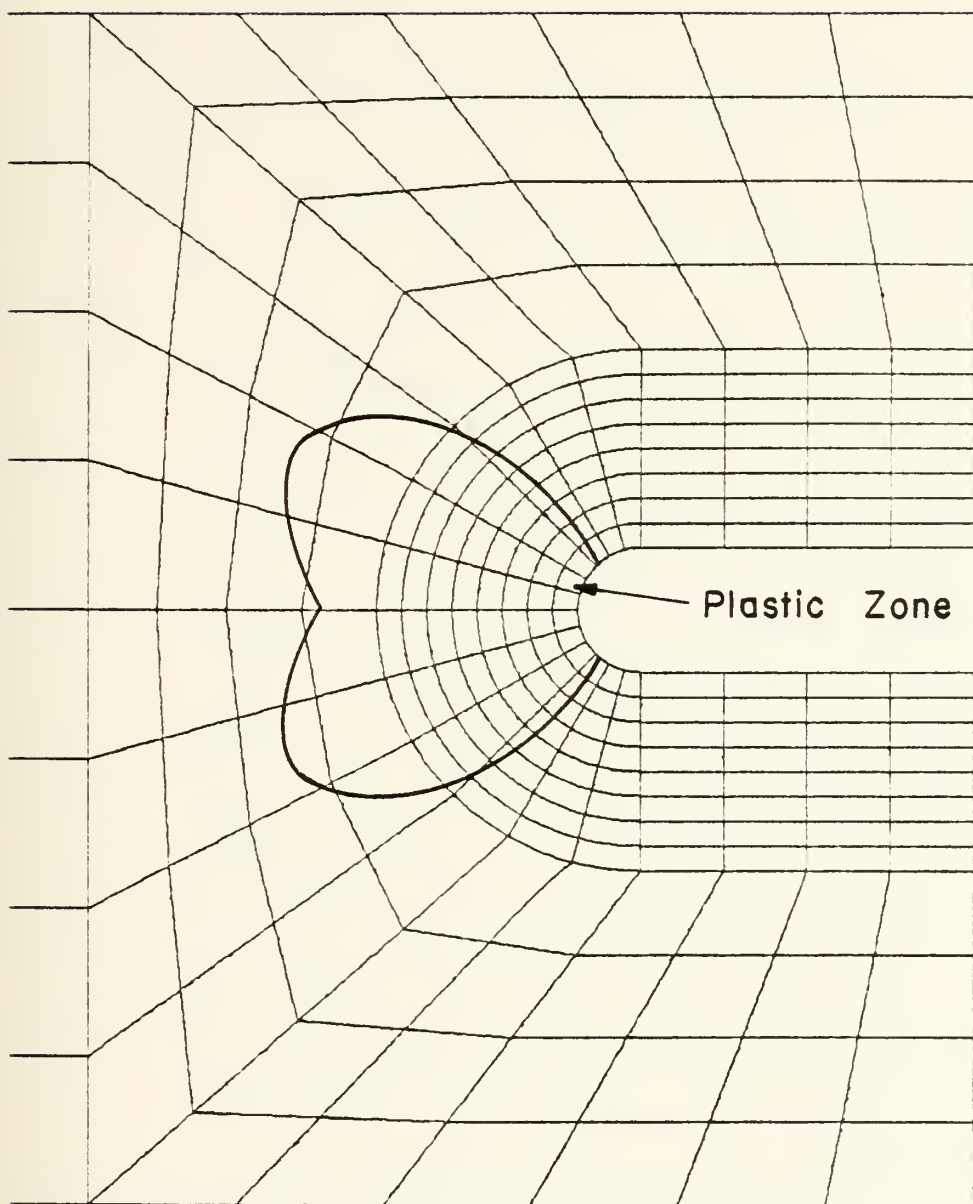


FIGURE 38

RIGID-PERFECTLY-PLASTIC INITIAL PLASTIC ZONE



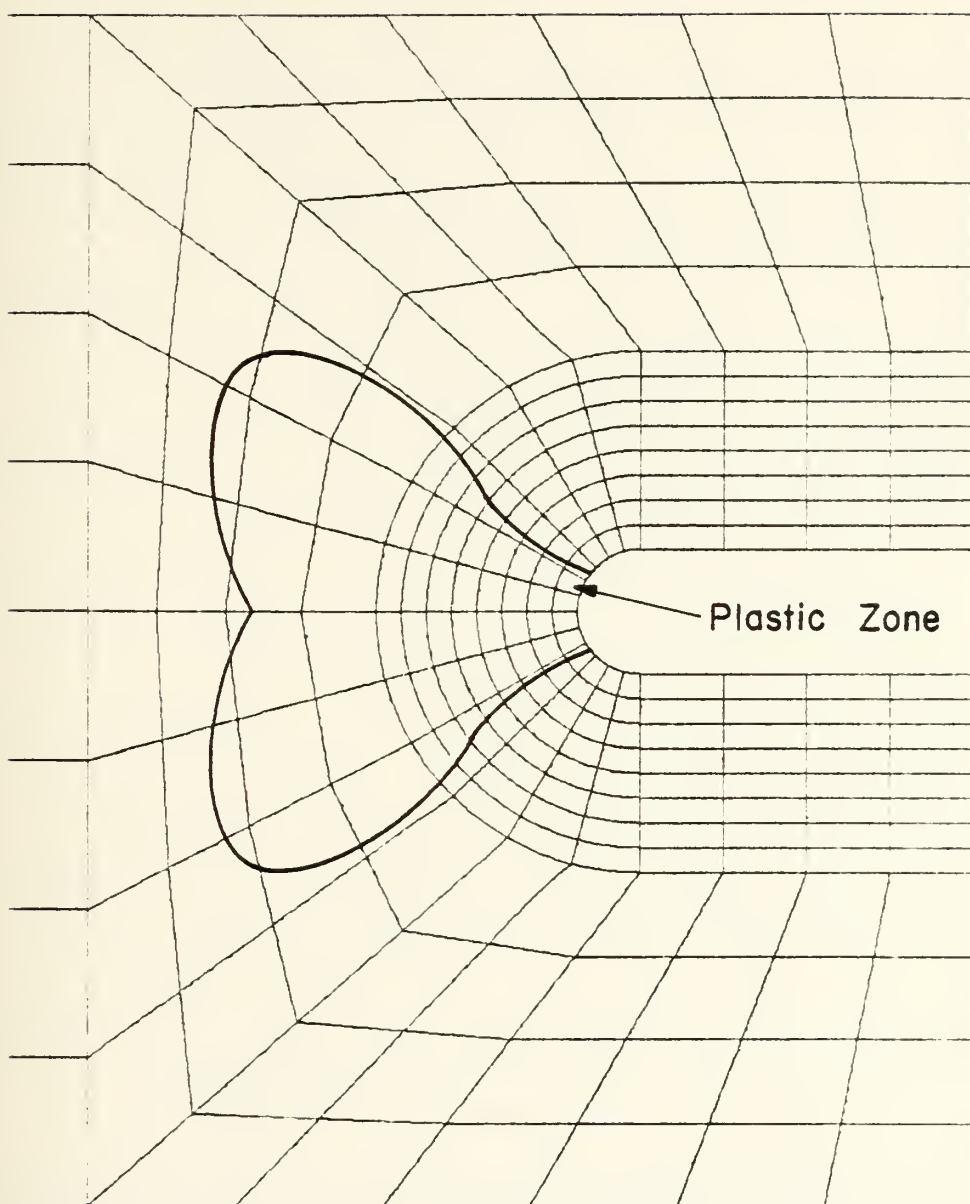


FIGURE 39

RIGID-PERFECTLY-PLASTIC INTERMEDIATE PLASTIC ZONE



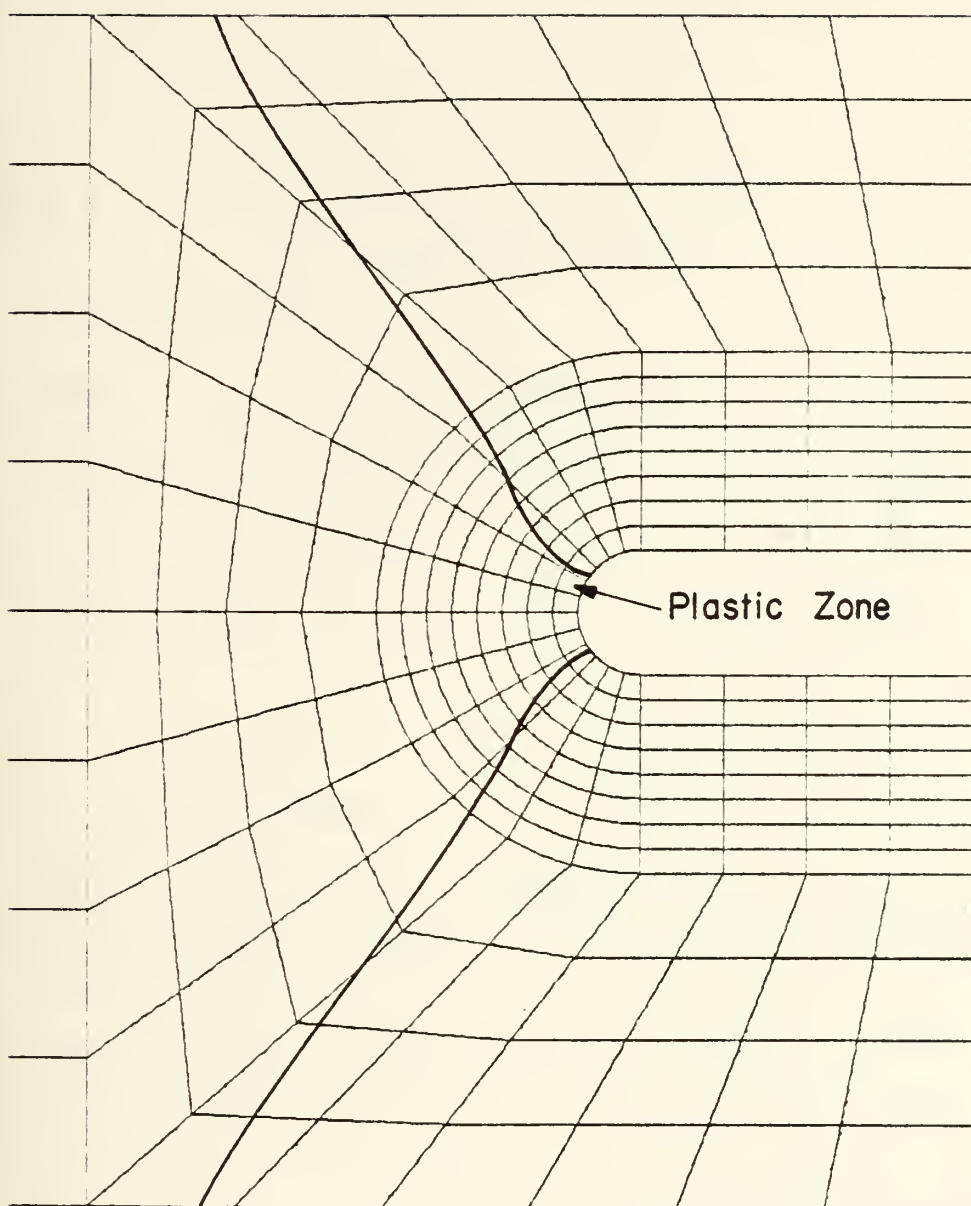


FIGURE 40

RIGID-PERFECTLY-PLASTIC FINAL PLASTIC ZONE





TABLE I. MTS AND REIHLE 5 GAGE TEST RESULTS

All Strains are  $10^{-6}$  in/in

## MTS Test Machine

LOAD lbs	STRAINS				
	$\epsilon_1$	$\epsilon_2$	$\epsilon_3$	$\epsilon_4$	$\epsilon_5$
1000	-3	421	814	1235	1638
2000	338	1003	1619	2279	2903
3000	960	1714	2431	3186	3908

## REIHLE Test Machine

LOAD lbs	STRAINS				
	$\epsilon_1$	$\epsilon_2$	$\epsilon_3$	$\epsilon_4$	$\epsilon_5$
1000	800	763	755	741	726
2000	1564	1540	1546	1543	1535
3000	2370	2334	2363	2377	2388



TABLE II. MTS SPECIMEN A TEST RESULTS

Cross-section =  $0.03975 \text{ in}^2$

Load, lbs.	Strain, $\epsilon, 10^{-6} \text{ in/in}$
256	615
503	1204
750	1801
1005	2423
1255	3042
1505	3665
1778	4352
2003	4925
2252	5587
2508	6355
2755	7365
2905	8230
2984	9045
3037	10150



TABLE III. MTS SPECIMEN B TEST RESULTS

Cross-section =  $0.03975 \text{ in}^2$

Load, lbs.	Strain, $\epsilon$ , $10^{-6} \text{ in/in}$
500	1250
750	1860
1000	2450
1250	3000
1500	3700
1750	4320
2000	4950
2250	5630
2500	6300
2750	7340
2900	8200
3000	9250
3050	10200
3100	11550
3125	12800



TABLE IV. MTS SPECIMEN C TEST RESULTS

Cross-section =  $0.03975 \text{ in}^2$

Load, lbs.	Strain, $\epsilon, 10^{-6} \text{ in/in}$
272	663
503	1220
762	1850
1008	2451
1231	3065
1506	3690
1755	4313
2000	4940
2255	5607
2503	6325
2750	7230
2900	8120
3000	9200
3060	10500
3100	11500
3125	12500





TABLE V.

## REIHLE SPECIMEN TEST RESULTS

Cross-section =  $0.12 \text{ in}^2$ 

LOAD lbs.	STRAIN, $\epsilon_1$ $10^{-6} \text{ in/in}$	STRAIN, $\epsilon_2$ $10^{-6} \text{ in/in}$
500	345	-115
1000	720	-240
1500	1114	-364
2000	1501	-492
2500	1895	-614
3000	2291	-745
3500	2705	-871
4000	3095	-999
4500	3515	-1126
5000	3912	-1255
5500	4332	-1384
6000	4750	-1520
6500	5188	-1647
7000	5595	-1784
7500	6075	-1923
8000	6649	-2103
8500	7385	-2375
9000	8663	-2888
9500	12245	-4435



TABLE VI.

 $\lambda = 0.2$  HOWLAND DATA

DISTANCE FROM HOLE in.	$\sigma_{\theta} / \sigma_{\infty}$
0.0	3.14
0.5	1.57
1.0	1.26
1.5	1.16
2.0	1.11
2.5	1.07
3.0	1.05
3.5	1.01
4.0	0.97

TABLE VII.

 $\lambda = 0.25$  HOWLAND DATA

DISTANCE FROM HOLE in.	$\sigma_{\theta} / \sigma_{\infty}$
0.000	3.23
0.422	1.75
0.828	1.36
1.234	1.22
1.641	1.13
2.047	1.07
2.453	1.04
2.859	0.97
3.063	0.95



TABLE VIII.

 $\lambda = 0.2$  FEA RESULTS - NODAL OUTPUT $\sigma_{\infty} = 5000$  psi

DISTANCE FROM HOLE, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.00	15938.2	351.7
0.25	10259.7	2375.8
0.50	7875.7	1783.5
0.75	6877.0	1673.1
1.00	6307.4	1334.7
1.25	5995.4	1087.2
1.50	5797.7	874.4
1.75	5633.9	700.5
2.00	5509.8	556.8

TABLE IX.

 $\lambda = 0.2$  FEA RESULTS - GAUSS OUTPUT $\sigma_{\infty} = 5000$  psi

DISTANCE FROM HOLE, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.00	15595.4	128.9
0.25	9482.6	2104.1
0.50	7953.8	1826.6
0.75	6783.7	1650.4
1.00	6316.1	1348.7
1.25	5975.6	1088.8
1.50	5795.8	890.1
1.75	5623.8	695.7
2.00	5499.9	554.9



TABLE X.

 $\lambda = 0.25$  FEA RESULTS - NODAL OUTPUT

DISTANCE FROM HOLE, in.	$\sigma_{\infty} = 4500$ psi	
	$\sigma_{\theta}$ ,psi	$\sigma_r$ ,psi
0.00	14745.5	329.8
0.25	9447.0	2181.1
0.50	7232.6	1599.3
0.75	6304.7	1473.5
1.00	5771.7	1139.6
1.25	5471.2	891.6
1.50	5266.9	671.0
1.75	5091.7	489.1
2.00	4940.2	354.5

TABLE XI.

 $\lambda = 0.25$  FEA RESULTS - GAUSS OUTPUT

DISTANCE FROM HOLE, in.	$\sigma_{\infty} = 4500$ psi	
	$\sigma_{\theta}$ ,psi	$\sigma_r$ ,psi
0.00	14422.8	121.2
0.25	8721.2	1927.1
0.50	7317.1	1639.7
0.75	6218.6	1452.4
1.00	5779.6	1151.5
1.25	5453.3	891.0
1.50	5267.1	677.3
1.75	5085.9	486.9
2.00	4937.9	343.8





TABLE XII. SHALLOW NOTCH FEA LINEAR RESULTS - NODAL

$$\sigma_n = 4225 \text{ psi}$$

DISTANCE FROM NOTCH, in.	$\sigma_\theta$ , psi	$\sigma_r$ , psi
0.00	11584.9	94.5
0.25	8966.1	1279.9
0.50	7335.6	1501.9
0.75	6347.3	1731.7
1.00	5655.0	1713.2
1.25	5184.1	1703.5
1.50	4832.4	1626.7
1.75	4564.7	1560.6
2.00	4361.1	1484.7

TABLE XIII. SHALLOW NOTCH FEA LINEAR RESULTS - GAUSS

$$\sigma_n = 4225 \text{ psi}$$

DISTANCE FROM NOTCH, in.	$\sigma_\theta$ , psi	$\sigma_r$ , psi
0.00	11530.2	11.5
0.25	8727.1	1177.7
0.50	7355.5	1504.8
0.75	6280.9	1713.6
1.00	5661.2	1720.4
1.25	5159.3	1699.7
1.50	4834.6	1634.2
1.75	4552.8	1561.7
2.00	4353.6	1486.0



TABLE XIV. DEEP NOTCH FEA LINEAR RESULTS - NODAL

$$\sigma_n = 4800 \text{ psi}$$

DISTANCE FROM NOTCH, in.	$\sigma_\theta$ , psi	$\sigma_r$ , psi
0.00	21142.8	755.2
0.25	11929.9	4484.3
0.50	8436.8	3530.4
0.75	6976.6	3524.4
1.00	6076.1	3152.2
1.25	5527.0	2883.4
1.50	5139.2	2623.4
1.75	4842.4	2412.0
2.00	4617.8	2234.2

TABLE XV. DEEP NOTCH FEA LINEAR RESULTS - GAUSS

$$\sigma_n = 4800 \text{ psi}$$

DISTANCE FROM NOTCH, in.	$\sigma_\theta$ , psi	$\sigma_r$ , psi
0.00	20368.1	314.0
0.25	10582.5	4035.2
0.50	8606.0	3611.8
0.75	6852.2	3501.9
1.00	6086.7	3172.2
1.25	5496.1	2884.7
1.50	5141.8	2640.0
1.75	4830.1	2421.2
2.00	4622.0	2238.8



TABLE XVI. SHALLOW NOTCH FEA NONLINEAR 60,000 lb LOAD

DISTANCE FROM NOTCH, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi	NO LOAD RESIDUALS	
			$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.00	80024.7	1094.5	-33411.4	439.1
0.25	81338.7	9523.3	-4250.0	-2690.1
0.50	81383.6	16052.6	9006.8	2188.8
0.75	65548.3	18993.2	2946.6	2133.4
1.00	58431.4	18799.6	1992.8	1796.7
1.25	52670.1	18347.9	1167.3	1501.6
1.50	49111.1	17477.1	825.7	1250.8
1.75	46080.7	16567.3	594.4	1046.7
2.00	43973.8	15667.8	471.6	885.0

TABLE XVII. SHALLOW NOTCH FEA NONLINEAR 65,000 lb LOAD

DISTANCE FROM NOTCH, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi	NO LOAD RESIDUALS	
			$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.00	80890.4	1004.6	-42092.3	224.7
0.25	82939.3	10229.5	-9796.3	-2782.0
0.50	83642.7	16054.6	6944.3	-711.2
0.75	74622.0	21475.5	7507.7	3214.2
1.00	64770.4	21171.3	3820.9	2838.7
1.25	57891.3	20593.6	2202.8	2419.0
1.50	53766.7	19537.0	1524.5	2018.5
1.75	50321.1	18474.8	1089.5	1710.1
2.00	47948.2	17421.4	853.7	1448.0



TABLE XVIII. SHALLOW NOTCH FEA NONLINEAR 70,000lb LOAD

DISTANCE FROM NOTCH, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi	NO LOAD RESIDUALS	
			$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.00	81957.9	704.5	-50670.6	63.9
0.25	82822.6	10668.8	-16176.6	-3285.1
0.50	84007.3	15514.4	-376.9	-2366.8
0.75	87017.4	22662.9	15132.6	2762.3
1.00	71318.0	23765.2	5681.1	3983.0
1.25	63762.9	23215.2	3763.1	3616.6
1.50	58941.5	21920.1	2669.9	3030.0
1.75	54897.6	20670.6	1865.4	2596.7
2.00	52177.0	19426.6	1448.2	2207.0

TABLE XIX. DEEP NOTCH FEA NONLINEAR 30,000 lb LOAD

DISTANCE FROM NOTCH, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi	NO LOAD RESIDUALS	
			$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.00	83521.0	2332.4	-14138.0	-725.2
0.25	58038.6	20971.3	5680.1	1402.5
0.50	43309.1	18640.5	385.7	824.6
0.75	34402.6	17839.1	198.3	468.9
1.00	30522.9	16076.1	125.0	300.5
1.25	27543.7	14574.8	88.9	215.3
1.50	25757.0	13328.7	67.3	160.7
1.75	24188.8	12195.0	53.5	126.3
2.00	23141.6	11265.7	44.9	100.9





TABLE XX. DEEP NOTCH FEA NONLINEAR 35,000 lb LOAD

DISTANCE FROM NOTCH,in.	$\sigma_{\theta}$ ,psi	$\sigma_r$ ,psi	NO LOAD RESIDUALS	
			$\sigma_{\theta}$ ,psi	$\sigma_r$ ,psi
0.00	79020.4	1258.1	-39063.2	-1909.2
0.25	78215.7	24598.9	16348.7	1113.1
0.50	50094.6	22675.1	105.7	1699.5
0.75	40516.0	21359.5	624.1	1002.8
1.00	35843.1	19156.9	380.6	716.7
1.25	32290.3	17299.1	257.8	521.7
1.50	30172.9	15782.1	203.5	405.9
1.75	28314.8	14410.8	156.9	319.5
2.00	27079.2	13294.4	133.7	261.5

TABLE XXI. DEEP NOTCH FEA NONLINEAR 40,000 lb LOAD

DISTANCE FROM NOTCH,in.	$\sigma_{\theta}$ ,psi	$\sigma_r$ ,psi	NO LOAD RESIDUALS	
			$\sigma_{\theta}$ ,psi	$\sigma_r$ ,psi
0.00	80929.5	835.4	-52900.9	-2121.6
0.25	89730.7	24933.8	20503.1	-2132.3
0.50	56572.6	26815.4	-610.4	2977.0
0.75	46984.6	24878.1	1468.2	1643.1
1.00	41400.6	22286.3	610.0	1216.6
1.25	37164.5	20068.3	555.9	892.7
1.50	34690.9	18287.2	438.5	710.5
1.75	32512.6	16672.2	330.7	564.8
2.00	31078.3	15368.8	282.0	470.8



TABLE XXII.

RIGID - PERFECTLY - PLASTIC RESULTS

$$\sigma_y = 73,000 \text{ psi}$$

DISTANCE FROM NOTCH, in.	$\sigma_\theta$ , psi	$\sigma_r$ , psi
0.00	73334.4	519.9
0.25	81780.5	28863.6
0.50	83252.6	29813.6
0.75	84074.1	37321.1
1.00	84103.1	40244.0
1.25	84091.8	42244.0
1.50	84051.5	43249.8
1.75	84005.0	44032.9
2.00	83958.1	43875.6

TABLE XXIII. EXPERIMENTAL DATA  $\lambda = 0.25$  HOLE LINEAR LOADING

$$\sigma_\infty = 10,749 \text{ psi}$$

DISTANCE FROM HOLE, in.	$\sigma_\theta$ , psi	$\sigma_r$ , psi
0.000	35321.0	49.5
0.125	24639.0	-1650.5
0.250	19809.5	669.5
0.375	17799.0	2311.5
0.500	16588.0	3422.0



TABLE XXIV. EXPERIMENTAL DATA SHALLOW NOTCH LINEAR LOADING

15,000 lb Load

DISTANCE FROM HOLE, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.000	26611.7	53.2
0.125	22295.0	1392.6
0.250	19999.3	2352.7
0.375	17277.5	2387.6
0.500	15798.7	2380.5
0.625	13679.1	1929.3

TABLE XXV. EXPERIMENTAL DATA SHALLOW NOTCH 60,000 lb LOAD

DISTANCE FROM NOTCH, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi	NO LOAD RESIDUALS	
			$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.000	81111.5	5551.0	-23842.0	4.0
0.125	79710.7	6735.8	-19837.0	-475.0
0.250	78112.0	8141.5	-16409.0	-1044.0
0.375	77189.3	11899.5	-12620.0	-1426.0
0.500	71207.2	10509.4	-12319.0	-5008.0
0.625	71436.4	19260.0	-10903.0	-6453.0
0.750	63045.8	16580.9		
0.875	59477.4	17748.6		
1.000	57146.4	19275.4		



TABLE XXVI. EXPERIMENTAL DATA SHALLOW NOTCH 65,000 lb LOAD

DISTANCE FROM NOTCH, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi	NO LOAD RESIDUALS	
			$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.000	83073.9	7058.0	-36745.0	-191.0
0.125	80957.6	6078.9	-32540.0	-387.0
0.250	79854.9	7468.1	-28256.0	-1554.0
0.375	79482.8	10878.9	-23914.0	-2381.0
0.500	79638.4	16634.0	-20192.0	-2686.0
0.625	77887.0	19867.6	-16300.0	-3392.0
0.750	72556.4	19706.7		
0.875	64863.2	17433.1		
1.000	61648.0	19034.1		

TABLE XXVII. EXPERIMENTAL DATA SHALLOW NOTCH 70,000 lb LOAD

DISTANCE FROM NOTCH, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi	NO LOAD RESIDUALS	
			$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.000	84748.9	8569.0	-50791.0	43.0
0.125	83492.9	9073.9	-46086.0	-736.0
0.250	84258.3	13718.2	-40774.0	-878.0
0.375	82846.8	13815.0	-34600.0	-1016.0
0.500	82524.6	16814.9	-30017.0	-1316.0
0.625	80446.4	17441.6	-24798.0	-1551.0
0.750	80455.5	23190.4		
0.875	76568.9	22928.7		
1.000	72264.1	23604.0		





TABLE XXVIII. EXPERIMENTAL DATA DEEP NOTCH LINEAR LOADING

15,000 lb Load

DISTANCE FROM NOTCH, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.000	45907.0	-45.0
0.125	35372.0	7693.0
0.250	30410.0	11671.0
0.375	26177.0	12395.0
0.500	20939.0	9879.0
0.625	18373.0	9060.0
0.750	16891.0	8984.0

TABLE XXIX. EXPERIMENTAL DATA DEEP NOTCH 30,000 lb LOAD

Elastic - Plastic

DISTANCE FROM NOTCH, in.	$\sigma_{\theta}$ , psi	$\sigma_r$ , psi
0.000	76156.6	2587.4
0.125	63996.2	6057.7
0.250	50047.3	10456.3
0.375	43992.3	14485.0
0.500	39748.6	16433.6
0.625	35030.8	15899.3
0.750	33286.8	16728.0



# APPENDIX A

## PSAP1 JCL

```

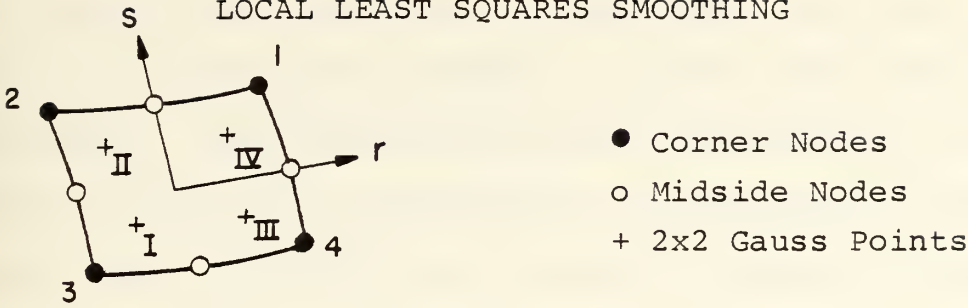
----- STANDARD JJB CARD -----
// EXEC FRTXCLGP
//FORT.SYSPRINT DD DUMMY
//FORT.SYSIN DD UNIT=3330,VOL=SER=DISK02,
// DSN=S2939.PSAP(PSAP),DISP=SHR,LABEL=(,,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(PLOT),
// DISP=SHR,LABEL=(,,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(INIT),
// DISP=SHR,LABEL=(,,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(ELER),
// DISP=SHR,LABEL=(,,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(SAPF),
// DISP=SHR,LABEL=(,,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(ADNA),
// DISP=SHR,LABEL=(,,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(AUXL),
// DISP=SHR,LABEL=(,,,IN)
// DD UNIT=3330,VOL=SER=DISK02,DSN=S2939.PSAP(ADPT),
// DISP=SHR,LABEL=(,,,IN)
// DD *
C *** MAIN PROGRAM ***
  DIMENSION ZZZ(3203),DISPD(5,3,800)
  CALL PSAP1(ZZZ,3203,DISPD,800)
  STOP
  END
***** DELIMITER CARD (/*) *****
//GO.FT10FO01 DD UNIT=SYSDA,DISP=(,PASS),
// SPACE=(CYL,(2,2)),DSN=88TEMP1
//GO.SYSIN DD *
***** INSERT PSA'1 DATA HERE *****
***** DELIMITER CARD (/*) *****

```



## APPENDIX B

### LOCAL LEAST SQUARES SMOOTHING



Two-Dimensional Isoparametric Element from ADINA [Ref. 4]

The local smoothing expression from Hinton and Campbell [Ref. 19] in ADINA coordinates becomes

$$\begin{Bmatrix} \tilde{\sigma}_1 \\ \tilde{\sigma}_2 \\ \tilde{\sigma}_3 \\ \tilde{\sigma}_4 \end{Bmatrix} = \begin{bmatrix} C & B & B & A \\ B & A & C & B \\ A & B & B & C \\ B & C & A & B \end{bmatrix} \times \begin{Bmatrix} \sigma_I \\ \sigma_{II} \\ \sigma_{III} \\ \sigma_{IV} \end{Bmatrix}$$

where  $A = 1 + \frac{\sqrt{3}}{2}$ ,  $B = -\frac{1}{2}$  and  $C = 1 - \frac{\sqrt{3}}{2}$ .

With  $\tilde{\sigma}_1$ ,  $\tilde{\sigma}_2$ ,  $\tilde{\sigma}_3$  and  $\tilde{\sigma}_4$  representing the smoothed corner node stresses and  $\sigma_I$ ,  $\sigma_{II}$ ,  $\sigma_{III}$ , and  $\sigma_{IV}$  as the unsmoothed stresses at the Gauss integration points, this expression can be written in an equivalent form.

$$\begin{Bmatrix} \tilde{\sigma}_3 \\ \tilde{\sigma}_4 \\ \tilde{\sigma}_1 \\ \tilde{\sigma}_2 \end{Bmatrix} = \begin{bmatrix} A & B & C & B \\ B & A & B & C \\ C & B & A & B \\ B & C & B & A \end{bmatrix} \times \begin{Bmatrix} \sigma_I \\ \sigma_{III} \\ \sigma_{IV} \\ \sigma_{II} \end{Bmatrix}$$



The midside node stress values may be obtained by averaging the values at the associated corner nodes, since the distribution of the smoothed stresses is linear along the sides of the element. Smoothed stress values obtained by this least squares method should subsequently be averaged to obtain unique values at nodal points shared by adjacent elements.





# APPENDIX C

## ADINA JCL

```

----- STANDARD JJB CARD -----
// EXEC FORTXCLG,REGION=2000K
//FORT.SYSPRINT DD DJMAY
//FORT.SYSIN DD *
    IMPLICIT REAL*8(A-H,O-Z)
    REAL A
    COMMON A(120010)
    MAX=120000
    CALL EXEC(MAX)
    STOP
    END
***** DELIMITER CARD (/*) *****
//LKED.JSDD DD DISP=SHR,DSN=MSS.S2939.ADINA
//LKED.SYSIN DD *
    INCLUDE USDD(LOADM)
    ENTRY MAIN
***** DELIMITER CARD (/*) *****
//GO.FT07F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT01F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT02F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT03F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.FT04F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.FT08F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT09F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT10F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=4000),SPACE=(CYL,(5,1))
//GO.FT11F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.FT12F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.FT13F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.FT56F001 DD UNIT=3330,VOL=SER=DISK01,
//  DSN=F0099.TEMP,DISP=SHR,LABEL=(, ,IN)
//GO.FT57F001 DD UNIT=SYSDA,DISP=(NEW,DELETE),
//  DCB=(RECFM=VBS,BLKSIZE=1000),SPACE=(CYL,(5,1))
//GO.SYSIN DD *
*****
***** INSERT ADINA DATA HERE *****
*****
    --- BLANK CARD ---
    --- BLANK CARD --- (TWO BLANK CARDS STOP EXEC)
***** DELIMITER CARD (/*) *****

```



# APPENDIX D PSAP1 LISTING

```

PSAP1      MAR 1981  AS MODIFIED BY LCDR M.J. KAISER
SUBROUTINE PSAP1 DOCUMENTATION

      DESCRIPTION OF INPUT DATA CARDS

      TITLE CARD - 80 ALPHANUMERIC CHARACTERS OF GRAPH TITLE INFORMATION
                   TO BE PRINTED ABOVE AND BELOW THE GRAPH. THE FIRST 40
                   CHARACTERS WILL FORM THE FIRST TITLE LINE. THE LAST 40
                   THE SECOND LINE.
APR1981
DDCU0020
DDCU0030
DDCU0040
DDCU0050
DDCU0060
DDCU0070
DDCU0080
DDCU0090
DDCU0100
DDCU0110
DDCU0120
DDCU0130
DDCU0140
DDCU0150
DDCU0160
DDCU0170
DDCU0180
DDCU0190
DDCU0200
DDCU0210
DDCU0220
DDCU0230
DDCU0240
DDCU0250
DDCU0260
DDCU0270
DDCU0280
DDCU0290
DDCU0300
DDCU0310
DDCU0320
DDCU0330
DDCU0340
DDCU0350
DDCU0360
DDCU0370
DDCU0380
DDCU0390
DDCU0400
DDCU0410
DDCU0420
DDCU0430
DDCU0440
DDCU0450
DDCU0460
DDCU0470
DDCU0480

      NAMELIST OPTION - CONTAINS VALUES TO VERIFY STORAGE IN BLANK
                       COMMON AND CONTROL VALUES NEEDED BY THE PROGRAM.

      THE FOLLOWING VALUES ARE INCLUDED---

NNDEST = ESTIMATE NUMBER OF GRID POINTS TO BE USED. VALUE MUST
          BE GREATER THAN OR EQUAL TO THE ACTUAL NUMBER OF GRID
          POINTS.
          ** DEFAULT = 200 **
NUDISP = 0 FOR NO DISPLACEMENT DATA IN X-DIRECTION.
          ** DEFAULT = 1 FOR DATA INCLUDING DISPLACEMENTS IN X-DIRECTION.
NVDISP = 0 FOR NO DISPLACEMENT DATA IN Y-DIRECTION.
          ** DEFAULT = 1 FOR DATA INCLUDING DISPLACEMENTS IN Y-DIRECTION.
NWDISP = 0 FOR NO DISPLACEMENT DATA IN Z-DIRECTION.
          ** DEFAULT = 1 FOR DATA INCLUDING DISPLACEMENTS IN Z-DIRECTION.

      KGEOM SPECIFIES SUBROUTINE AND CORRESPONDING METHOD OF INPUT
      FOR MODEL GEOMETRY.
      KGEOM = 1 FOR USER SUPPLIED SUBROUTINE - GEOM1
              = 2 FOR USER DEVELOPED TO READ ADINA GEOMETRY DATA - MAR 77
              = 9 FOR USER SUPPLIED SUBROUTINE - GEOM2
              = 9 FOR SAP IV DATA DECK INPUT SUBROUTINE - GEOM9.
          ** DEFAULT = 9 **
      KDATA SPECIFIES SUBROUTINE AND CORRESPONDING METHOD OF INPUT
      FOR DISPLACEMENT DATA.
      KDATA = 1 FOR SUBROUTINE DATA1 TO READ IN DISPLACEMENT DATA
              -- SUPPLIED BY THE USER.

```



```

= 5 FOR SUBROUTINE DATA5 TO READ IN DISPLACEMENT DATA
-- SUPPLIED BY THE USER.
= 9 FOR SUBROUTINE DATA9 TO READ SAP IV DATA.
** DEFAULT = 9 **
NVALUS - NOT USED AT NPS ----- ALLOW DEFAULT

** DE=FAULT = 0 **
IRESEQ - NOT USED AT NPS ----- ALLOW TO DEFAULT
** DEFAULT = 1 **
KPLOT SPECIFIES THE TYPE OF OUTPUT DEVICE TO BE USED.
KPLOT = 1 FOR CALCOMP. RESEARCH CENTER USE ONLY
= 2 FOR LANGLEY ONLY.
= 3 FOR LRC USE ONLY
= 4 FOR LRC USE ONLY
** DEFAULT = 1 **
YSPACE = SPACE BETWEEN PLOTS IN Y DIRECTION (INCHES) WHEN
MULTIPLE PLOTS ARE PRODUCED. YSPACE/2.0 IS SPACE
BETWEEN TITLE BLOCK AND PLOT.
** DEFAULT = 2.0 **
PSIZE = PAPER SIZE IN X DIRECTION, USED IN SCALING OF
PLOTS TO INSURE THIS DIMENSION IS NOT EXCEEDED.
** DEFAULT = 9.0 **
IDCASE = 0 FOR NO TITLE CARD PRECEDING
DECKS OF DISPLACEMENT VALUES.
= 1 FOR TITLE CARD PRECEDING
DECKS OF DISPLACEMENT VALUES.
** DEFAULT = 0 **

```

MODEL GEOMETRY IS NOW INPUT IN ONE OF THE FOLLOWING FORMS,  
 DEPENDING ON THE VALUE OF KGEOM SPECIFIED IN NAMELIST OPTION.

```

USE IF KGEOM = 1
CALL SUBROUTINE GEOM1 WHICH READS ADINA GEOMETRY DATA

USE IF KGEOM = 2
CALL SUBROUTINE GEOM2 WHICH IS PREPARED BY THE USER TO
READ GEOMETRY DATA.

```

```

USE IF KGEOM = 9
CALL SUBROUTINE GEOM9 WHICH READS SAP IV GEOMETRY DATA.

```

DOCU0490  
 DOCU0500  
 DOCU0510  
 DOCU0520  
 DOCU0530  
 DOCU0540  
 DOCU0550  
 DOCU0560  
 DOCU0570  
 DOCU0580  
 DOCU0590  
 DOCU0600  
 DOCU0610  
 DOCU0620  
 DOCU0630  
 DOCU0640  
 DOCU0650  
 DOCU0660  
 DOCU0670  
 DOCU0680  
 DOCU0690  
 DOCU0700  
 DOCU0710  
 DOCU0720  
 DOCU0730  
 DOCU0740  
 DOCU0750  
 DOCU0760  
 DOCU0770  
 DOCU0780  
 DOCU0790  
 DOCU0800  
 DOCU0810  
 DOCU0820  
 DOCU0830  
 DOCU0840  
 DOCU0850  
 DOCU0860  
 DOCU0870  
 DOCU0880  
 DOCU0890  
 DOCU0900  
 DOCU0910  
 DOCU0920  
 DOCU0930  
 DOCU0940  
 DOCU0950  
 DOCU0960





[illegible]

**CASE IDENTIFICATION CARD.**

THIS CARD IS OMITTED IF IDCASE=0 IS SPECIFIED IN &OPTION  
IF PRESENT, THIS CARD CONTAINS ANY DESIRED ALPHANUMERIC  
INFORMATION IN COLUMNS 1-80 WILL NOT APPEAR ON PLOT BUT WILL  
APPEAR ON PRINTOUT ABOVE DISPLACEMENT DATA

DATA TO BE PLOTTED IS NOW INPUT IN ONE OF THE FOLLOWING FORMS,  
DEPENDING ON THE VALUE OF KDATA SPECIFIED IN NAMELIST OPTION.

USE IF KDATA = 1  
CALL SUBROUTINE DATA WHICH IS PREPARED BY THE USER

```

USE IF KDATA = 5
CALL SUBROUTINE DATAS WHICH IS PREPARED BY THE USER

```

```
USE IF KDATA = 9
```

CALL SUBROUTINE DATA9 WHICH READS SAP IV DISPLACEMENT DATA.  
A DISPLACEMENT DATA DECK CAN BE PREPARED FOR ADINA IN A  
FORMAT COMPATIBLE WITH DATA9.

NAMELIST PICT - CONTAINS VALUES NEEDED TO GENERATE PLOTS.

THE FOLLOWING VALUES ARE INCLUDED---

KKHORZ = INTEGER DESIGNATING HORIZONTAL AXIS OF VIEWING PLANE,  
WHERE 1=X, 2=Y, 3=Z.

```
**  
** DEFAULT = 1  
**
```

DELTA = 1 +  
INTEGER DESIGNATING VERTICAL AXIS OF VIEWING PLANE,  
WHERE 1=X, 2=Y, 3=Z.

```

**
**      I = 2
**      DEFAULT = 2

```

PHI = ANGULAR ROTATION OF MODEL ABOUT ITS X-AXIS, IN DEGREES (MUST BE TAKEN THIRD).

```
** ** *  
** ** *  
** ** *
```

THETA =  $\frac{U}{U_0}$  + ANGULAR ROTATION OF MODEL ABOUT ITS Y-AXIS, IN DEGREES (MJST BE TAKEN SECOND).

```

**
** DEFAULT = 0.0
**

```

PSI = ANGULAR ROTATION OF MODEL ABOUT ITS Z-AXIS, IN DEGREES





```

(MUST BE TAKEN FIRST).
** DEFAULT = 0.0 **
NEWFR = 1 FOR FRAME CHANGE BEFORE PLOT IS MADE.
      (A FRAME CHANGE RESETS THE Y-ORIGIN PAST PREVIOUS PLOT
      BY YSPACE AND X-ORIGIN AT 0.0)
NEWFR.NE.1 FOR NO FRAME CHANGE BEFORE PLOTTING
** DEFAULT = 1 **
ISCALE = 1 FOR INTERNAL ORIGIN LOCATION AND SCALING.
      = 2 FOR USER SPECIFIED ORIGIN AND SCALING.
      = 0 FOR NO SCALE CHANGE. (I.E. USE SAME SCALE AS PREVIOUS
      PLOT) THIS IS USEFUL IN AN ASSEMBLY GRAPH WHERE IT IS
      NECESSARY TO EXAMINE A MESH IN SECTIONS WITHOUT LOSING
      PERSPECTIVE. ISCALE CANNOT BE ZERO ON THE FIRST PLOT.
** DEFAULT = 1 **
PLOTSZ = MAXIMUM DIMENSION DESIRED ON COMPLETED PLOT.
      (USED FOR SCALING IF ISCALE=1)
PLOTSZ SCALES THE PLOT PRIOR TO ROTATION. IF ROTATION
CAUSES THE PLOT TO EXCEED PAPER WIDTH (PSIZE), IT IS
RESCALED AND THE PLOT SIZE IS REDUCED ACCORDINGLY.
** DEFAULT = 10.0 **
XORGN = X-LOCATION OF PLOT ORIGIN (USED IF ISCALE = 2).
** DEFAULT = 0.0 **
YORGN = Y-LOCATION OF PLOT ORIGIN (USED IF ISCALE = 2).
** DEFAULT = 0.0 **
PSCALE = MODEL SIZE REDUCTION FACTOR, PSCALE = ACTUAL MODEL
      SIZE/DESIRED PLOT SIZE (USED IF ISCALE = 2).
** DEFAULT = 1.0 **
NOTAT = 0 FOR NO NUMBERING ON PLOTS.
      = 1 FOR NUMBERING OF GRID POINTS.
      = 2 FOR NUMBERING OF ELEMENTS.
** DEFAULT = 0 **
XLHT = HEIGHT OF INTEGERS SPECIFIED BY NOTAT, IN INCHES.
** DEFAULT = 0.15 **
KDISP = 0 FOR UNDEFORMED PLOT.
      = 1 FOR DEFORMED PLOT.
      = 2 FOR EXPLODED PLOT.
      = 3 FOR DISPLACEMENTS REPRESENTED BY VECTORS.
** DEFAULT = 0 **
IDMAG = 1 FOR DIRECT SCALING OF DATA BY DMAGS.
      = 2 FOR SCALING OF DATA TO A MAX. VALUE OF DMAGS.
** DEFAULT = 2 **
DMAGS = MAGNIFICATION OF DISPLACEMENTS (IF KDISP=1).
      = REDUCTION FACTOR OF ELEMENTS (IF KDISP=2).
** DEFAULT = 1.0 **
KSYMXY = 1 FOR SYMMETRY ABOUT X-Y PLANE.
** DEFAULT = 0 **
KSYMXZ = 1 FOR SYMMETRY ABOUT X-Z PLANE.
** DEFAULT = 0 **

```

CC



```

KSYMZY = 1 FOR SYMMETRY ABOUT Y-Z PLANE.
** DEFAULT = 0 **
XXMAX, YYMAX, ZZMAX, XXMIN, YYMIN, ZZMIN LOCATE CUTTING PLANES
PARALLEL TO PRINCIPAL (X-Y, X-Z, Y-Z) PLANES
TO LIMIT PLOT.
** DEFAULT XXMAX=YYMAX=ZZMAX=1.0E+20 **
** DEFAULT XXMIN=YYMIN=ZZMIN=-1.0E+20 **
NDMAX ** MAXIMUM GRID PT. TO BE INCLUDED IN PLOT.
NDMIN ** DEFAULT = 9999999999 **
** MINIMUM GRID PT. TO BE INCLUDED IN PLOT.
NELMAX ** MAXIMUM ELEMENT NUMBER TO BE INCLUDED IN PLOT.
** DEFAULT = 9999999999 **
NELMIN ** MINIMUM ELEMENT NUMBER TO BE INCLUDED IN PLOT.
** DEFAULT = 0 **
CODE SPECIFIES CONTROL OPTION AFTER PLOT IS COMPLETE.
CODE = 0, LAST PLOT, EXIT FROM PROGRAM.
= 1, READ ANOTHER NAMELIST PICT.
= 2, READ A NEW SET OF DISPLACEMENT DATA, INCLUDING A
= 3, CASE IDENTIFICATION CARD IF PRESENT.
** READ A COMPLETE NEW SET OF INPUT DATA,
INCLUDING A TITLE CARD.
** DEFAULT = 0 **

```

THE ABOVE COMPRISES A COMPLETE BASIC SET OF INPUT DATA IF  
 CODE = 0 IN &PICT. FOR CODE = 1, 2, OR 3, ADDITIONAL SECTIONS OF  
 THE BASIC DECK MUST BE REPEATED. THE DECK MUST END WITH  
 NAMELIST &PICT HAVING CODE = 0.

SUBROUTINE PSAPI IS A MODIFICATION TO NAVAL POSTGRADUATE  
 SCHOOL THESIS BY LT. O. M. LOSH, DECEMBER 1976. MODIFICATION  
 INCLUDED SAP IV 8-21 NODE BRICK ELEMENTS, BOUNDARY ELEMENTS AND  
 ADIVA TRUSS, PLANE, BRICK, BEAM ELEMENTS, AND OTHER MINOR  
 IMPROVEMENTS.

MODIFIED BY ADRIAN E. KIBLER, JR.  
 LT USN  
 NAVAL POSTGRADUATE SCHOOL  
 MONTEREY, CA.  
 JAN - JUN 1977

DOCU1930  
 DOCU1940  
 DOCU1950  
 DOCU1960  
 DOCU1970  
 DOCU1980  
 DOCU1990  
 DOCU2000  
 DOCU2010  
 DOCU2020  
 DOCU2030  
 DOCU2040  
 DOCU2050  
 DOCU2060  
 DOCU2070  
 DOCU2080  
 DOCU2090  
 DOCU2100  
 DOCU2110  
 DOCU2120  
 DOCU2130  
 DOCU2140  
 DOCU2150  
 DOCU2160  
 DOCU2170  
 DOCU2180  
 DOCU2190  
 DOCU2200  
 DOCU2210  
 DOCU2220  
 DOCU2230  
 DOCU2240  
 DOCU2400  
 DOCU2410  
 DOCU2420  
 DOCU2430  
 DOCU2440  
 DOCU2450  
 DOCU2460  
 DOCU2470  
 DOCU2480  
 DOCU2490  
 DOCU2500  
 DOCU2510  
 DOCU2520  
 DOCU2530  
 DOCU2540  
 DOCU2550









```

SUBROUTINE PSAP1(ZZZ,NZ,DISPD,NON)
*      *      *      *      *      *      *      *      *      *      *
** THIS IS THE MAIN SUBROUTINE WHICH CALLS OTHER SUBROUTINES
*      *      *      *      *      *      *      *      *      *      *
      INTEGER NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT
      COMMON/CDATA/NTIME,NTLC
      COMMON/CONTRL/ KGEOM,KDATA,KPLOT,KSYMXY,KSYMxz,KSYMZY,NOTAT,XLHT,
      1KHORZ,KVERT,PHI,THETA,PSI,NEWFR,ISCALE,PLOTSZ,XORGN,YORGN,
      2PSCALE,KDISP,DMAG,KODE
      COMMON/LIMITS/ XXMAX,YYMAX,ZZMAX,XXMIN,YYMIN,ZZMIN,NDMAX,NDMIN,
      1NELMAX,NELMIN
      COMMON/CORGN/ YPMAX,YSPACE,PSIZE
      COMMON/GLOOP/ ILOOP
      COMMON/ABLK/ A(3,3)
      COMMON/SAVEV/ DMAGS,DMAG
      COMMON/KOUNT/ NNODE,NNDIST,NUDISP,NVDISP,NWDISP
      COMMON/VALUES/ NVALUS
      COMMON/CASEID/ IDCASE
      DIMENSION ZZZ(VZ),DISPD(5,3,NON),ABCD1(10),ABCD2(10),ABCD3(10),
      1ABCD4(10)
      NAMELIST/PICT/ KHORZ,KVERT,PHI,THETA,PSI,NEWFR,ISCALE,
      1PLOTSZ,XORGN,YORGN,PSCALE,NOTAT,KDISP,DMAG,DMAGS,KODE,
      2KSYMXY,KSYMxz,XXMAX,YYMAX,ZZMAX,XXMIN,
      3YYMIN,ZZMIN,NDMAX,NDMIN,NELMAX,NELMIN,XLHT

```

```

PSAP0010
PSAP0020
PSAP0030
PSAP0040
PSAP0050
PSAP0060
PSAP0070
PSAP0080
PSAP0090
PSAP0100
PSAP0110
PSAP0120
PSAP0130
PSAP0140
PSAP0150
PSAP0160
PSAP0170
PSAP0180
PSAP0190
PSAP0200
PSAP0210
PSAP0220
PSAP0230
PSAP0240
PSAP0250
PSAP0260
PSAP0270
PSAP0280
PSAP0290
PSAP0300

```





```

C *** TO ZERO NODE AND ELEMENT SUMMATION COUNTERS
C
C      ILOOP = 0
C      NNODE = 0
C      YPMAX=0.0
C
C *** TO DEFINE THE ORIGIN AND OPEN PLOTTING DATA SETS
C
C      CALL CALCMP
C      CONTINUE
C      REWIND 10
C      WRITE(6,18)
C      FORMAT(1H1)
C
C *** TO READ TITLE CARD FOR RUN
C
C      READ(5,9004,END=999) (ABCD1(I), I=1,10), (ABCD2(I), I=1,10)
C      FORMAT(20A4)
C      WRITE(6,9006) (ABCD1(I), I=1,10), (ABCD2(I), I=1,10)
C      FORMAT(///,20X,20A4///)
C      CALL INITIAL
C
C *** TO PLOT THE TITLE CARD AT THE BEGINNING OF THE PLOT
C
C      CALL CALPLT(0.0,0.0,0.62,3)
C      CALL CALPLT(0.0,0.0,0.2)
C      CALL CALPLT(9.0,0.0,0.2)
C      CALL NOTATE(0.8,0.41,0.21,ABCD1,0.0,40)
C      CALL NOTATE(0.8,0.10,0.21,ABCD2,0.0,40)
C      CALL CALPLT(0.0,1.62+YSPACE/2.0,-3)
C
C *** TO SET POINTERS FOR BLANK COMMON STORAGE ZZZ SUBROUTINES)
C *** (WITH INTEGER NAMES OF ARRAYS USED IN CALLED SUBROUTINES)
C
C      NUMPT = 1
C      XPT = NUMPT+NNDEST
C      YPT = XPT+NNDEST
C      ZPT = YPT+NNDEST
C      UPT = ZPT+NNDEST
C      IF(NUDISP.EQ.0) VPT = UPT+1
C      IF(NUDISP.NE.0) VPT = UPT+NNDEST
C      IF(NVDISP.EQ.0) WPT = VPT+1
C      IF(NVDISP.NE.0) WPT = VPT+NNDEST
C      IF(NWDISP.EQ.0) NEND = WPT+1-1
C      IF(NWDISP.NE.0) NEND = WPT+NNDEST-1
C      WRITE(6,15) NEND
C      FORMAT(///,20X,'BLANK COMMON STORAGE ZZZ REQUIRES AT LEAST ',I6,

```

```

PSAP0310
PSAP0320
PSAP0330
PSAP0340
PSAP0350
PSAP0360
PSAP0370
PSAP0380
PSAP0390
APR1981
PSAP0410
PSAP0420
PSAP0430
PSAP0440
PSAP0450
PSAP0460
PSAP0470
PSAP0480
PSAP0490
PSAP0500
PSAP0510
PSAP0520
PSAP0530
PSAP0540
PSAP0550
APR1981
APR1981
APR1981
APR1981
APR1981
PSAP0630
PSAP0640
PSAP0650
PSAP0660
PSAP0670
PSAP0680
PSAP0690
PSAP0700
PSAP0710
PSAP0720
PSAP0730
PSAP0740
PSAP0750
PSAP0760
PSAP0770
PSAP0780
PSAP0790

```



```

1 LOCATIONS FOR THIS CASE'////)
1 IF(KGEOM.EQ.1) CALL GEOM1
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
1 IF(KGEOM.EQ.2) CALL GEOM2
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
1 IF(KGEOM.EQ.9) CALL GEOM3
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
1 CALL PNTOUT(1)
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
600 CONTINUE
1 IF(IDCASE.EQ.3) GO TO 650
READ(5,9004,END=999) (ABCD3(I),I=1,10),(ABCD4(I),I=1,10)
WRITE(6,9006) (ABCD3(I),I=1,10),(ABCD4(I),I=1,10)
650 CONTINUE
CALL ZERO
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
1 IF(KDATA.EQ.1) CALL DATA1
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
1 IF(KDATA.EQ.5) CALL DATA5
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
1 IF(KDATA.EQ.9) CALL DATA9
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
2DISP,VON)
1 IF (NUDISP.EQ.0.AND.NVDISP.EQ.0.AND.NWDISP.EQ.0) GO TO 700
CALL PNTOUT(2)
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
700 CONTINUE
1 IF(KPLOT.EQ.4.AND.ILOOP.NE.0) GO TO 6000
WRITE(6,1000)
FORMAT(//)
READ(5,PICT)
WRITE(6,PICT)
6000 CONTINUE
CALL DSCALE
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
CALL BOUND
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
1 IF(ISCAL.NE.0) CALL ROTAT
CALL PLOTX
1(ZZZ(NUMPT),ZZZ(XPT),ZZZ(YPT),ZZZ(ZPT),ZZZ(UPT),ZZZ(VPT),ZZZ(WPT))
1 ILOOP=ILOOP+1
GO TO (700,600),CODE
C *** TO PLOT TITLE ON TOP OF GRAPH IF CODE = 3
C *** TO PLOT TITLE ON TOP AND CLOSE PLOTTING DATA SETS IF CODE = 0
C
CALL CALPLT(0.0,YPMAX+YSPACE/2.0,-3)
CALL CALPLT(0.0,1.0,3)

```

PSAP0800  
 PSAP0810  
 PSAP0820  
 PSAP0830  
 PSAP0840  
 PSAP0850  
 PSAP0860  
 PSAP0870  
 PSAP0880  
 PSAP0890  
 PSAP0900  
 PSAP0910  
 PSAP0920  
 PSAP0930  
 PSAP0940  
 PSAP0950  
 PSAP0960  
 PSAP0970  
 PSAP0980  
 PSAP0990  
 PSAP1000  
 PSAP1010  
 PSAP1020  
 PSAP1030  
 PSAP1040  
 PSAP1050  
 PSAP1060  
 PSAP1070  
 PSAP1080  
 PSAP1090  
 PSAP1100  
 PSAP1110  
 PSAP1120  
 PSAP1130  
 PSAP1140  
 PSAP1150  
 PSAP1160  
 PSAP1170  
 PSAP1180  
 PSAP1190  
 PSAP1200  
 PSAP1210  
 PSAP1220  
 PSAP1230  
 PSAP1240  
 PSAP1250  
 APR1981  
 APR1981



```

CALL CALPLT(0.0,1.62,2)
CALL CALPLT(9.0,1.62,2)
CALL NOTATE(0.8,1.31,.21,ABCD1,0.0,40)
CALL NOTATE(0.8,1.0,.21,ABCD2,0.0,40)
CALL CALPLT(0.0,1.62+YSPACE,-3)
ILOOP=0
IF(KODE.EQ.3) GO TO 500
WRITE(6,9008)
FORMAT(//,5X,'TERMINATION NORMAL DUE TO KODE = 0')
CALL PSTOP
RETURN
999 CALL ERROR(2)
RETURN
END
SUBROUTINE PLOTX(NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C * * * * *
C *** FOR GENERATING PLOTS.
C *** CALLED BY PSAPI
C * * * * *
COMMON/CONTROL/ KGEOM,KDATA,KPLOT,KSYMXY,KSYMZX,KSYMZY,NOTAT,XLHT,
1KHORZ,KVERT,PHI,THETA,PSI,NEWFR,ISCALE,PLOTSZ,XORGN,YORGN,
2PSCALE,KDISP,DYAG,KODE
COMMON/LIMITS/ XXMAX,YYMAX,ZZMAX,XXMIN,YYMIN,ZZMIN,NDMAX,NDMIN,
1NELMAX,NELMIN
COMMON/XYZLIM/ XYZMAX(3),XYZMIN(3)
COMMON/CORGN/ YPMAX,YSPACE,PSIZE
COMMON/GLOOP/ ILOOP
COMMON/ABLK/ A(3,3)
COMMON/KOUNT/ NNODE,NNDIST,NUDISP,NWDISP
COMMON/PDELS/ DELX,DELY
COMMON/NUMPT/ NUMPT(1),XPT(1),YPT(1),ZPT(1),UPT(1),VPT(1),WPT(1)
COMMON/NDIM/ NDIM(1),X(20),Y(20),Z(20),XDISP(20),YDISP(20),
1ZDISP(20),XROT(20),YROT(20),XP(23),YP(23)
C *** TO MAKE ALL GRID POINT NUMBERS NEGATIVE
C
C
C
DO 50 I=1,NNODE
IF(NUMPT(I).GT.0) NUMPT(I)=-NUMPT(I)
50 CONTINUE
C *** TO MAKE FRAME CHANGE IF NEWFR = 1
C *** NO FRAME CHANGE ON FIRST PLOT AFTER NAMELIST OPTION
C
C
C
YMOVE=0.0

```

```

APR1981
APR1981
APR1981
APR1981
PSAPI340
PSAPI350
PSAPI360
PSAPI370
APR1981
PSAPI390
PSAPI400
PSAPI410
PSAPI420
PLOT0010
PLOT0020
PLOT0030
PLOT0040
PLOT0050
PLOT0060
PLOT0070
PLOT0080
PLOT0090
PLOT0100
PLOT0110
PLOT0120
PLOT0130
PLOT0140
PLOT0150
PLOT0160
PLOT0170
PLOT0180
PLOT0190
PLOT0200
PLOT0210
PLOT0220
PLOT0230
PLOT0240
PLOT0250
PLOT0260
PLOT0270
PLOT0280
PLOT0290
PLOT0300
PLOT0310
PLOT0320
PLOT0330
PLOT0340

```





```

IF(ILOOP.EQ.0) GO TO 70
IF(NEWFR.EQ.1) YMOVE=YPMAX+YSPACE
70 CALL CALPLT(0.0,YMOVE,-3)
GO TO (710,710,703,710),KPLOT
703 CONTINUE
C
710 CONTINUE
IF(ISCAL.EQ.0) DELX=0.0
IF(ISCAL.EQ.1) DELY=0.0
IF(ISCAL.EQ.2) CALL XYSCAL
CALL CALPLT(XORGN,YORGN,-3)
XSHIFT = 0.0
YSHIFT = 0.0
ZSHIFT = 0.0
YPMAX=-1.0E20
C
*** LOOPS TO ACCOUNT FOR SYMMETRY
C
C
ZSIGN = +1.0
DO 500 II=1,2
IF(II.EQ.2.AND.KSYMXY.NE.1) GO TO 500
IF(II.EQ.2.AND.KSYMXY.EQ.1) ZSIGN = -1.0
YSIGN = +1.0
DO 510 JJ=1,2
IF(JJ.EQ.2.AND.KSYMxz.NE.1) GO TO 510
IF(JJ.EQ.2.AND.KSYMxz.EQ.1) YSIGN = -1.0
XSIGN = +1.0
DO 520 KK=1,2
IF(KK.EQ.2.AND.KSYMZY.NE.1) GO TO 520
IF(KK.EQ.2.AND.KSYMZY.EQ.1) XSIGN = -1.0
C
*** TO DETERMINE PROJECTED COORDINATES OF ELEMENTS
C
C
REWIND 10
CONTINUE
100 READ(10,END=1000) NEND,NUMEL,(NODE(J),J=1,NEND)
IF(NUMEL.LT.NELMIN.OR.NUMEL.GT.NELMAX) GO TO 100
DO 10 I=1,NEND
ND = NODE(I)
IF(NODE(I).EQ.0) GO TO 10
C
*** TO MAKE GRID POINT NUMBERS CONNECTED BY ELEMENTS POSITIVE
C
NUMPT(ND) = IABS(NUMPT(ND))
IF(NUMPT(ND).LT.NDMIN.OR.NUMPT(ND).GT.NDMAX) GO TO 100
10 CONTINUE
I = KXORZ
J = KVERT
DO 20 N=1,NEND

```

```

PLOT0350
PLOT0360
APR1981
PLOT0380
PLOT0390
APR1981
PLOT0410
PLOT0420
PLOT0430
PLOT0440
APR1981
PLOT0460
PLOT0470
PLOT0480
PLOT0490
PLOT0500
PLOT0510
PLOT0520
PLOT0530
PLOT0540
PLOT0550
PLOT0560
PLOT0570
PLOT0580
PLOT0590
PLOT0600
PLOT0610
PLOT0620
PLOT0630
PLOT0640
PLOT0650
PLOT0660
PLOT0670
PLOT0680
PLOT0690
PLOT0700
PLOT0710
PLOT0720
PLOT0730
PLOT0740
PLOT0750
PLOT0760
PLOT0770
PLOT0780
PLOT0790
PLOT0800
PLOT0810
PLOT0820

```





```

IF(NODE(N).EQ.0) GO TO 20
ND = NODE(N)
IF(XPT(ND).GT.XXMAX) GO TO 100
IF(XPT(ND).LT.XXMIN) GO TO 100
IF(YPT(ND).GT.YYMAX) GO TO 100
IF(YPT(ND).LT.YYMIN) GO TO 100
IF(ZPT(ND).GT.ZZMAX) GO TO 100
IF(ZPT(ND).LT.ZZMIN) GO TO 100
XDISP(N) = 0.0
YDISP(N) = 0.0
ZDISP(N) = 0.0
IF(KDISP.EQ.1.AND.NUDISP.NE.0) XDISP(N) = UPT(ND)
IF(KDISP.EQ.1.AND.NVDISP.NE.0) YDISP(N) = VPT(ND)
IF(KDISP.EQ.1.AND.NWDISP.NE.0) ZDISP(N) = WPT(ND)
X(N) = XSIGN*(XPT(ND)+XDISP(N)*DMAG+XSHIFT)/PSCALE
Y(N) = YSIGN*(YPT(ND)+YDISP(N)*DMAG+YSHIFT)/PSCALE
Z(N) = ZSIGN*(ZPT(ND)+ZDISP(N)*DMAG+ZSHIFT)/PSCALE
20 CONTINUE
IF(KDISP.EQ.2) CALL XPLOD(NEND,X,Y,Z,NODE)
XCENT = 0.0
YCENT = 0.0
FND=0.0
DO 25 N=1,NEND
IF(NODE(N).EQ.0) GO TO 25
XROT(N) = A(I,1)*X(N)+A(I,2)*Y(N)+A(I,3)*Z(N)
YROT(N) = A(J,1)*X(N)+A(J,2)*Y(N)+A(J,3)*Z(N)
IF(N.GT.8) GO TO 24
FND=FND+1.0
XCENT = XCENT+XROT(N)
YCENT = YCENT+YROT(N)
24 CONTINUE
XROT(N) = XROT(N)+DELX
YROT(N) = YROT(N)+DELY
IF(YROT(N).GT.YPMAX) YPMAX=YROT(N)
25 CONTINUE
IF(NOTAT.NE.2) GO TO 29
XCENT = XCENT/FND-(6.0/7.0)*XLHT
YCENT = YCENT/FND-XLHT/2.0
XCENT = XCENT+DELX
YCENT = YCENT+DELY
AL = NUMEL
29 CONTINUE
IF(NOTAT.EQ.2) CALL CALNUM(XCENT,YCENT,XLHT,AL,0.0,-1)
C *** TO PLOT ELEMENTS
C
IF(NEND.EQ.2) GO TO 280
IF(NEND.EQ.4) GO TO 300

```



```

IF(NEND.EQ.8) GO TO 320
IF(NEND.EQ.12) GO TO 340
IF(NEND.EQ.20) GO TO 340
CALL ERROR(4)
C
C ***TO PLOT 2 NODE ELEMENT
C
280 CONTINUE
CALL CALPLT(XROT(1),YROT(1),3)
CALL CALPLT(XROT(2),YROT(2),2)
GO TO 430
C
C *** TO PLOT 3 AND 4 NODE PLANE ELEMENT
C
300 CONTINUE
CALL CALPLT(XROT(1),YROT(1),3)
DO 305 NP=2,NEND
CALL CALPLT(XROT(NP),YROT(NP),2)
305 CONTINUE
CALL CALPLT(XROT(1),YROT(1),2)
GO TO 430
C
C *** TO PLOT 8 NODE 3-D BRICK
C
320 CONTINUE
LP=1
DO 330 NP=2,6,4
NP2=NP+2
CALL CALPLT(XROT(LP),YROT(LP),3)
DO 325 MP=NP,NP2
CALL CALPLT(XROT(MP),YROT(MP),2)
325 CONTINUE
CALL CALPLT(XROT(LP),YROT(LP),2)
LP=LP+4
330 CONTINUE
NP4=NP+4
DO 335 NP=1,4
CALL CALPLT(XROT(NP),YROT(NP),3)
CALL CALPLT(XROT(NP4),YROT(NP4),2)
335 CONTINUE
GO TO 430
C
C *** TO PLOT VARIABLE 4-8 NODE PLANE AND 8-20 NODE BRICK ELEMENTS
C
340 CONTINUE
LP=1
KP=8
DO 365 NP=2,6,4

```

```

PLOT1310
PLOT1320
PLOT1330
PLOT1340
PLOT1350
PLOT1360
PLOT1370
PLOT1380
APR1981
APR1981
PLOT1410
PLOT1420
PLOT1430
PLOT1440
PLOT1450
APR1981
PLOT1470
APR1981
PLOT1490
APR1981
PLOT1510
PLOT1520
PLOT1530
PLOT1540
PLOT1550
PLOT1560
PLOT1570
PLOT1580
APR1981
PLOT1600
APR1981
PLOT1620
APR1981
PLOT1640
PLOT1650
PLOT1660
PLOT1670
APR1981
APR1981
PLOT1700
PLOT1710
PLOT1720
PLOT1730
PLOT1740
PLOT1750
PLOT1760
PLOT1770
PLOT1780

```



```

NP2=NP+2
CALL CALPLT(XROT(LP),YROT(LP),3)
DO 345 MP=NP,NP2
  KP=KP+1
  N=2
  CALL CALWH(XP(1),YP(1))
  XP(2)=XROT(MP)
  YP(2)=YROT(MP)
  XP(3)=XROT(KP)
  YP(3)=YROT(KP)
  IF(NODE(KP).NE.0) CALL CURVE(XP,YP,N)
  CALL CALINE(XP,YP,N)
  CONTINUE
  KP=KP+1
  N=2
  CALL CALWH(XP(1),YP(1))
  XP(2)=XROT(LP)
  YP(2)=YROT(LP)
  XP(3)=XROT(KP)
  YP(3)=YROT(KP)
  IF(NODE(KP).NE.0) CALL CURVE(XP,YP,N)
  CALL CALINE(XP,YP,N)
  LP=LP+4
  IF(NEND.EQ.12) GO TO 430
  CONTINUE
  DO 390 NP=1,4
    NP4=NP+4
    KP=NP+16
    N=2
    XP(1)=XROT(NP)
    YP(1)=YROT(NP)
    XP(2)=XROT(NP4)
    YP(2)=YROT(NP4)
    XP(3)=XROT(KP)
    YP(3)=YROT(KP)
    IF(NODE(KP).NE.0) CALL CURVE(XP,YP,N)
    CALL CALINE(XP,YP,N)
    CONTINUE
  390 CONTINUE
  430 GO TO 100
  1000 CONTINUE
  IF(KDISP.NE.3) GO TO 650
  600 CONTINUE
C *** TO PLOT VECTORS AT GRID POINTS
C
C
DO 601 ND=1,NNODE
IF(NUMPT(ND).LE.0) GO TO 601

```

PLOT1790  
 APR1981  
 PLOT1810  
 PLOT1820  
 PLOT1830  
 APR1981  
 PLOT1850  
 PLOT1860  
 PLOT1870  
 PLOT1880  
 PLOT1890  
 APR1981  
 PLOT1910  
 PLOT1920  
 PLOT1930  
 APR1981  
 PLOT1950  
 PLOT1960  
 PLOT1970  
 PLOT1980  
 PLOT1990  
 APR1981  
 PLOT2010  
 PLOT2020  
 PLOT2030  
 PLOT2040  
 PLOT2050  
 PLOT2060  
 PLOT2070  
 PLOT2080  
 PLOT2090  
 PLOT2100  
 PLOT2110  
 PLOT2120  
 PLOT2130  
 PLOT2140  
 APR1981  
 PLOT2160  
 PLOT2170  
 PLOT2180  
 PLOT2190  
 PLOT2200  
 PLOT2210  
 PLOT2220  
 PLOT2230  
 PLOT2240  
 PLOT2250  
 PLOT2260



```

IF(NUMPT(ND),LT,NDMIN,OR,NUMPT(ND),GT,NDMAX) GO TO 601
IF(XPT(ND),GT,XVZMAX(1)) GO TO 601
IF(XPT(ND),LT,XVZMIN(1)) GO TO 601
IF(YPT(ND),GT,YVZMAX(2)) GO TO 601
IF(YPT(ND),LT,YVZMIN(2)) GO TO 601
IF(ZPT(ND),GT,ZVZMAX(3)) GO TO 601
IF(ZPT(ND),LT,ZVZMIN(3)) GO TO 601
X(1) = XSIGN*(XPT(ND)+XSHIFT)/PSCALE
Y(1) = YSIGN*(YPT(ND)+YSHIFT)/PSCALE
Z(1) = ZSIGN*(ZPT(ND)+ZSHIFT)/PSCALE
XDISP(1) = 0.0
YDISP(1) = 0.0
ZDISP(1) = 0.0
IF(NVDISP.NE.0) XDISP(1) = UPT(ND)
IF(NVDISP.NE.0) YDISP(1) = VPT(ND)
IF(NWDISP.NE.0) ZDISP(1) = WPT(ND)
X(2) = XSIGN*(XPT(ND)+XDISP(1)*DMAG+XSHIFT)/PSCALE
Y(2) = YSIGN*(YPT(ND)+YDISP(1)*DMAG+XSHIFT)/PSCALE
Z(2) = ZSIGN*(ZPT(ND)+ZDISP(1)*DMAG+ZSHIFT)/PSCALE
I = KQRTZ
J = KVERT
DO 605 N=1,2
  XROT(N) = A(I,1)*X(N)+A(I,2)*Y(N)+A(I,3)*Z(N)
  YROT(N) = A(J,1)*X(N)+A(J,2)*Y(N)+A(J,3)*Z(N)
  XROT(N) = XROT(N)+DELX
  YROT(N) = YROT(N)+DELY
CONTINUE
605 XARW = 0.06
  YARW = XARW/3.0
  CALL GARROW(XROT(1),YROT(1),XROT(2),YROT(2),1,XARW,YARW)
CONTINUE
601 CONTINUE
650 CONTINUE
520 CONTINUE
510 CONTINUE
500 CONTINUE
C *** TO PLOT NODE POINT NUMBERS
C
C
IF(NOTAT.EQ.1) CALL NOLET(NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
CALL CALPLT(-XORGN,-YORGN,-3)
C *** TO MAKE ALL GRID POINT NUMBERS POSITIVE AGAIN
C
C
DO 1100 I=1,NNDDE
  NUMPT(I)=IASS(NUMPT(I))
CONTINUE
1100 RETURN
END

```

PLOT2270  
 PLOT2280  
 PLOT2290  
 PLOT2300  
 PLOT2310  
 PLOT2320  
 PLOT2330  
 PLOT2340  
 PLOT2350  
 PLOT2360  
 PLOT2370  
 PLOT2380  
 PLOT2390  
 PLOT2400  
 PLOT2410  
 PLOT2420  
 PLOT2430  
 PLOT2440  
 PLOT2450  
 PLOT2460  
 PLOT2470  
 PLOT2480  
 PLOT2490  
 PLOT2500  
 PLOT2510  
 PLOT2520  
 PLOT2530  
 PLOT2540  
 PLOT2550  
 PLOT2560  
 PLOT2570  
 PLOT2580  
 PLOT2590  
 PLOT2600  
 PLOT2610  
 PLOT2620  
 PLOT2630  
 PLOT2640  
 PLOT2650  
 APR1981  
 PLOT2670  
 PLOT2680  
 PLOT2690  
 PLOT2700  
 PLOT2710  
 PLOT2720  
 PLOT2730  
 PLOT2740













```

C *** TO DETERMINE SCALE FACTOR FOR MODEL GEOMETRY.
C *** CALLED BY PLOTX
C *
C * * * * *
COMMON/CONTRL/ KGEOM,KDATA,KPLOT,KSYMXY,KSYMXX,KSYMZY,KSYMZY,NOTAT,XLHT,
1KHORZ,KVERT,PHI,THETA,PSI,NEWFR,ISCALE,PLOTSZ,XORGN,YORGN,
2PSCALE,KDISP,DMAG,KODE
COMMON/XYZLIM/ XYZMAX(3),XYZMIN(3)
COMMON/CORGN/ YPMAX,YSPACE,PSIZE
COMMON/ABLK/ A(3,3)
COMMON/PDELS/ DELX,DELY
I = KHORZ
J = KVERT
DMAX = 0.0
DO 5 N=1,3
VDUM = ABS(XYZMAX(N)-XYZMIN(N))
IF(VDUM.GT.DMAX) DMAX = VDUM
5 CONTINUE
PSCALE = DMAX/PLOTSZ
DO 10 L=1,2
DO 10 M=1,2
DO 10 N=1,2
X = XYZMIN(1)
IF(L.EQ.2) X = XYZMAX(1)
Y = XYZMIN(2)
IF(M.EQ.2) Y = XYZMAX(2)
Z = XYZMIN(3)
IF(N.EQ.2) Z = XYZMAX(3)
XRROT = A(I,1)*X+A(I,2)*Y+A(I,3)*Z
YRROT = A(J,1)*X+A(J,2)*Y+A(J,3)*Z
IF(L*M*N.NE.1) GO TO 30
20 CONTINUE
XRMIN = XRROT
XRMAX = XRROT
YRMIN = YRROT
YRMAX = YRROT
30 CONTINUE
XRMAX = XRROT
XRMIN = XRROT
YRMAX = YRROT
YRMIN = YRROT
10 CONTINUE
XRMAX=XRMIN)
IF(XR/PSCALE.GT.PSIZE) PSCALE=XR/PSIZE
XRMAX = XRMAX/PSCALE
YRMAX = YRMAX/PSCALE
XRMIN = XRMIN/PSCALE
YRMIN = YRMIN/PSCALE

```



```

DELX = -XRMIN
DELY = -YRMIN
XORGN = (PSIZE-XR/PSCALE)/2.0
YORGN = 0.0
RETURN
END
SUBROUTINE XPLDD(NEND,X,Y,Z,NODE)
C * * * * *
C *** FOR GENERATING EXPLODED PLOTS. ***
C *** CALLED BY PLOTX ***
C * * * * *
COMMON/CONTRL/ KGEOM,KDATA,KPLOT,KSYMXY,KSYMXYZ,KSYMZY,NOTAT,XLHT,
1KHORZ,KVERT,PHI,THETA,PSI,NEWFR,ISCALE,PLOTSZ,XORGN,YORGN,
2PSCALE,KDISP,DIMAG,KODE
DIMENSION X(20),Y(20),Z(20),NODE(20)
C * * * * *
C ** TO CALCULATE THE INCENTER OF TRIANGLES
C
IF(NODE(4).EQ.0) NEND=3
IF(NEND.NE.3) GO TO 20
10 CONTINUE
A = SQRT((X(2)-X(3))**2+(Y(2)-Y(3))**2+(Z(2)-Z(3))**2)
B = SQRT((X(1)-X(3))**2+(Y(1)-Y(3))**2+(Z(1)-Z(3))**2)
C = SQRT((X(1)-X(2))**2+(Y(1)-Y(2))**2+(Z(1)-Z(2))**2)
AC1 = A/(A+B+C)
AC2 = B/(A+B+C)
AC3 = C/(A+B+C)
XOC = AC1*X(1)+AC2*X(2)+AC3*X(3)
YOC = AC1*Y(1)+AC2*Y(2)+AC3*Y(3)
ZOC = AC1*Z(1)+AC2*Z(2)+AC3*Z(3)
GO TO 190
20 CONTINUE
C * * * * *
C ** TO CALCULATE THE CENTROID OF RODS,BARS, AND QUADS
C
XOC = 0.0
YOC = 0.0
ZOC = 0.0
FND=0.0
DO 100 I=1,NEND
IF(NODE(I).EQ.0) GO TO 100
IF(I.GT.8) GO TO 101
FND=FND+1.0
XOC = XOC+X(I)

```















PLOT5650  
 PLOT5660  
 PLOT5670  
 PLOT5680  
 PLOT5690  
 PLOT5700  
 PLOT5710  
 PLOT5720  
 PLOT5730  
 PLOT5740  
 PLOT5750  
 PLOT5760  
 PLOT5770  
 PLOT5780  
 PLOT5790  
 PLOT5800  
 PLOT5810  
 PLOT5820  
 PLOT5830  
 APR1981  
 PLOT5850  
 PLOT5860  
 PLOT5870

```

DO 500 I=1, NNJDE
  IF(NUMPT(I)).LE.0) GO TO 500
  IF(NUMPT(I)).LT.NDMIN.OR.NUMPT(I).GT.NDMAX) GO TO 500
  IF(XPT(I).GT.XYZMAX(1)) GO TO 500
  IF(XPT(I).LT.XYZMIN(1)) GO TO 500
  IF(YPT(I).GT.XYZMAX(2)) GO TO 500
  IF(YPT(I).LT.XYZMIN(2)) GO TO 500
  IF(ZPT(I).GT.XYZMAX(3)) GO TO 500
  IF(ZPT(I).LT.XYZMIN(3)) GO TO 500
  X = (XPT(I)+XSHIFT)/PSCALE
  Y = (YPT(I)+YSHIFT)/PSCALE
  Z = (ZPT(I)+ZSHIFT)/PSCALE
  XROT = A(I,1)*X+A(I,2)*Y+A(I,3)*Z
  YROT = A(JJ,1)*X+A(JJ,2)*Y+A(JJ,3)*Z
  XL = XROT+XLHT/2.0
  YL = YROT+XLHT/2.0
  XL = XL+DELY
  YL = YL+DELY
  AL = NUMPT(I)
  CALL CALNUM(XL,YL,XLHT,AL,0.0,-1)
  CONTINUE
500 RETURN
END
  
```



```

C      SUBROUTINE INITIAL
C      *      *      *      *      *      *      *      *      *      *
C      *** TO SET UP VALUES FOR CONTROL PARAMETERS
C      *** CALLED BY PSAPI
C      *      *      *      *      *      *      *      *      *      *
C      COMMON/CDATA/NTIME,NTLC
C      COMMON/CONTRL/ KGEOM,KDATA,KPLDT,KSYMXY,KSYMXYZ,KSYMZY,NOTAT,XLHT,
C      1KHORZ,<VERT,PHI,THETA,PSI,NEWFR,ISCALE,PLOTSZ,XORGN,YORGN,
C      2PSCALE,KDISP,DMAG,KODE
C      COMMON/LIMITS/ XXMAX,YYMAX,ZZMAX,XXMIN,YYMIN,ZZMIN,NDMAX,NDMIN,
C      1NELMAX,NELMIN
C      COMMON/CONTRN/ YPMAX,YSPACE,PSIZE
C      COMMON/SAVEV/ DMAGS,IDMAGS
C      COMMON/KOUNT/ NNODE,NNDEST,NUDISP,NVDISP,NWDISP
C      COMMON/SEQUCE/ IRESEQ
C      COMMON/VALUES/ NVALUS
C      COMMON/CASEID/ IDCASE
C      NAMELIST/OPTION/ NNDEST,NUDISP,NVDISP,NWDISP,
C      1KGEOM,KDATA,NVALUS,IRESEQ,KPLOT,YSPACE,PSIZE,IDCASE
C      *** DESCRIPTION OF VALUES IN &OPTION GIVEN IN SUBROUTINE DCCMNT
C      *      *      *      *      *      *      *      *      *      *
C      *** TO SET DEFAULT VALUES FOR &OPTION
C      NNDEST = 200
C      NUDISP=0
C      NVDISP=0
C      NWDISP=0
C      KGEOM=9
C      KDATA=9
C      NTIME=0
C      NVALUS = 0
C      IRESEQ = 1
C      KPLOT = 1
C      YSPACE=2.0
C      PSIZE=9.0
C      IDCASE = 0
C      *** TO SET DEFAULT VALUES FOR &PICT
C      KHORZ = 1

```





```

KVERT = 2
PHI = 0.0
THETA = 0.0
PSI = 0.0
NEWFR = 1
ISCALE = 1
PLOTSZ = 10.0
XORGN = 0.0
YORGN = 0.0
PSCALE = 1.0
NOTAT = 0
XLHT = 0.1400
KDISP = 0
IDMAG = 2
DMAGS = 1.0
KSYMXY = 0
KSYMxz = 0
KSYMZY = 0
XXMAX = 1.0E20
YYMAX = 1.0E20
ZZMAX = 1.0E20
XXMIN = -1.0E20
YYMIN = -1.0E20
ZZMIN = -1.0E20
NDMAX = 9999999
NDMIN = 0
NELMAX = 9999999
NELMIN = 0
CODE = 0
READ(5,OPTION,END=999)
WRITE(6,9010)
FORMAT(////)
WRITE(6,OPTION)
WRITE(6,9010)
RETURN
CALL ERROR(3)
RETURN
END
SUBROUTINE BOUND(NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C * * * * *
C ** TO DETERMINE MAXIMUM DIMENSIONAL LIMITS OF BODY FOR USE
C ** IN SCALING PLJTS
C ** CALLED BY PSAPI
C * * * * *
INIT0490
INIT0500
INIT0510
INIT0520
INIT0530
INIT0540
INIT0550
INIT0560
INIT0570
INIT0580
INIT0590
INIT0600
INIT0610
INIT0620
INIT0630
INIT0640
INIT0650
INIT0660
INIT0670
INIT0680
INIT0690
INIT0700
INIT0710
INIT0720
INIT0730
INIT0740
INIT0750
INIT0760
INIT0770
INIT0780
INIT0790
INIT0800
INIT0810
INIT0820
INIT0830
INIT0840
INIT0850
INIT0860
INIT0870
INIT0880
INIT0890
INIT0900
INIT0910
INIT0920
INIT0930
INIT0940
INIT0950
INIT0960

```



```

COMMON/CONTRL/ KGEOM, KDATA, KPLOT, KSYMXY, KSYMxz, KSYMZY, NOTAT, XLHT,
1KHORZ, KVERT, PHI, THETA, PSI, NEWFR, ISCALE, PLOTSZ, XORGN, YORGN,
2PSCALE, KDISP, DMAG, KODE
COMMON/LIMITS/ XXMAX, YYMAX, ZZMAX, XXMIN, YYMIN, ZZMIN, NDMAX, NDMIN,
1NELMAX, NELMIN
COMMON/XYZLIM/ XYZMAX(3), XYZMIN(3)
COMMON/KOUNT/ NNODE, NNDIST, NUDISP, NVDISP, NWDISP
DIMENSION NUMPT(1), XPT(1), YPT(1), ZPT(1), UPT(1), VPT(1), WPT(1)
DIMENSION NODE(20)
DO 5 I=1, 3
XYZMIN(I) = +1.0E20
XYZMAX(I) = -1.0E20
5 CONTINUE
REWIND 10
100 CONTINUE
READ(10, END=1000) NEND, NJMEL, (NODE(I), I=1, NEND)
IF(NJMEL.LT.NELMIN.OR.NUMEL.GT.NELMAX) GO TO 100
DO 10 I=1, NEND
ND = NODE(I)
IF(NODE(I).EQ.0) GO TO 10
IF(NUMPT(ND).LT.NDMIN.OR.NUMPT(ND).GT.NDMAX) GO TO 100
10 CONTINUE
DO 20 I=1, NEND
IF(NODE(I).EQ.0) GO TO 20
ND = NODE(I)
IF(XPT(ND).GT.XXMAX) GO TO 20
IF(XPT(ND).LT.XXMIN) GO TO 20
IF(YPT(ND).GT.YYMAX) GO TO 20
IF(YPT(ND).LT.YYMIN) GO TO 20
IF(ZPT(ND).GT.ZZMAX) GO TO 20
IF(ZPT(ND).LT.ZZMIN) GO TO 20
IF(XPT(ND).GT.XYZMAX(1)) XYZMAX(1) = XPT(ND)
IF(XPT(ND).LT.XYZMIN(1)) XYZMIN(1) = XPT(ND)
IF(YPT(ND).GT.XYZMAX(2)) XYZMAX(2) = YPT(ND)
IF(YPT(ND).LT.XYZMIN(2)) XYZMIN(2) = YPT(ND)
IF(ZPT(ND).GT.XYZMAX(3)) XYZMAX(3) = ZPT(ND)
IF(ZPT(ND).LT.XYZMIN(3)) XYZMIN(3) = ZPT(ND)
20 CONTINUE
GO TO 100
1000 CONTINUE
DO 300 I=1, 3
IF(I.EQ.1.AND.KSYMZY.NE.1) GO TO 300
IF(I.EQ.2.AND.KSYMxz.NE.1) GO TO 300
IF(I.EQ.3.AND.KSYMXY.NE.1) GO TO 300
XYZBIG = ABS(XYZMAX(I))
IF(ABS(XYZMIN(I)).GT.XYZBIG) XYZBIG = ABS(XYZMIN(I))
XYZMAX(I) = XYZBIG
XYZMIN(I) = -XYZBIG

```

```

INIT0970
INIT0980
INIT0990
INIT1000
INIT1010
INIT1020
INIT1030
INIT1040
INIT1050
INIT1060
INIT1070
INIT1080
INIT1090
INIT1100
INIT1110
INIT1120
INIT1130
INIT1140
INIT1150
INIT1160
INIT1170
INIT1180
INIT1190
INIT1200
INIT1210
INIT1220
INIT1230
INIT1240
INIT1250
INIT1260
INIT1270
INIT1280
INIT1290
INIT1300
INIT1310
INIT1320
INIT1330
INIT1340
INIT1350
INIT1360
INIT1370
INIT1380
INIT1390
INIT1400
INIT1410
INIT1420
INIT1430
INIT1440

```



```

300 CONTINUE
RETURN
END
SUBROUTINE ZER3D(NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
* * * * *
*** INITIALIZES ALL DISPLACEMENTS TO ZERO.
*** CALLED BY PSAPI
* * * * *
COMMON/KOUNT/ NNODE,NNEST,NUDISP,NVDISP,NWDISP
DIMENSION NUMPT(1),XPT(1),YPT(1),ZPT(1),UPT(1),VPT(1),WPT(1)
IF(NUDISP.EQ.0) GO TO 200
DO 150 I=1,NUDISP
UPT(I) = 0.0
150 CONTINUE
200 CONTINUE
IF(NVDISP.EQ.0) GO TO 300
DO 250 I=1,NVDISP
VPT(I) = 0.0
250 CONTINUE
300 CONTINUE
IF(NWDISP.EQ.0) GO TO 400
DO 350 I=1,NWDISP
WPT(I) = 0.0
350 CONTINUE
400 CONTINUE
RETURN
END
SUBROUTINE PN3DUT(IOUT,NJAPT,XPT,YPT,ZPT,UPT,VPT,WPT)
* * * * *
*** FOR PRINTED OUTPUT OF INFORMATION IN BLANK COMMON - ZZZ
*** CALLED BY PSAPI
* * * * *
COMMON/KOUNT/ NNODE,NNEST,NUDISP,NVDISP,NWDISP
DIMENSION NUMPT(1),XPT(1),YPT(1),ZPT(1),UPT(1),VPT(1),WPT(1)
GO TO (1000,2000), IOUT
1000 CONTINUE
*** FOR OUTPUT OF GEOMETRY INFORMATION
C C C

```

```

INIT1450
INIT1460
INIT1470
INIT1480
INIT1490
INIT1500
INIT1510
INIT1520
INIT1530
INIT1540
INIT1550
INIT1560
INIT1570
INIT1580
INIT1590
INIT1600
INIT1610
INIT1620
INIT1630
INIT1640
INIT1650
INIT1660
INIT1670
INIT1680
INIT1690
INIT1700
INIT1710
INIT1720
INIT1730
INIT1740
INIT1750
INIT1760
INIT1770
INIT1780
INIT1790
INIT1800
INIT1810
INIT1820
INIT1830
INIT1840
INIT1850
INIT1860
INIT1870
INIT1880
INIT1890
INIT1900
INIT1910
INIT1920

```



```

WRITE(6,16)
16 FORMAT(///,5X,'GRID POINT INFORMATION',///)
WRITE(6,17)
17 FORMAT(5X,'RESEQUENCED',4X,'USER INPUT',
15X,'GRID POINT',5X,'GRID POINT',
25X,'NUMBER',9X,'NUMBER',13X,'X',14X,'Y',14X,'Z',/)
DO 30 I=1,NNODE
WRITE(6,18) I,NUMPT(I),XPT(I),YPT(I),ZPT(I)
18 FORMAT(2X,I10,5X,I10,3X,3E15.4)
30 CONTINUE
WRITE(6,19)
19 FORMAT(///,5X,'ELEMENT INFORMATION - WITH RESEQUENCED GRID POINTS',
1,///)
9008 FORMAT(1X,'RESEQUENCED',4X,'USER INPUT',25X,'GRID POINTS',/
11X,'ELEMENT',8X,'ELEMENT',/
21X,'NUMBER',9X,'NUMBER',7X,'1 2 3 4 5 6 7
3 8 9 10 11 12 13 14 15 16 17 18 19 20',/)
REWIND 10
I = 0
35 CONTINUE
I = I+1
READ(10,END=999) NEND,NUMEL,(NODE(J),J=1,NEND)
IF(NEND.EQ.12) GO TO 40
WRITE(6,9010) I,NUMEL,(NODE(J),J=1,NEND)
9010 FORMAT(1X,I4,11X,I4,9X,20I5)
GO TO 35
40 WRITE(6,9010) I,NUMEL,(NODE(J),J=1,4),(NODE(J),J=9,12)
GO TO 35
2000 CONTINUE
C *** FOR OUTPUT OF DISPLACEMENT DATA
C
C
WRITE(6,210)
210 FORMAT(///,5X,'DISPLACEMENTS TO BE PLOTTED',///)
WRITE(6,17)
DO 230 I=1,NNODE
U = 0.0
IF(NUDISP.NE.0) U = UPT(I)
V = 0.0
IF(NVDISP.NE.0) V = VPT(I)
W = 0.0
IF(NWDISP.NE.0) W = WPT(I)
WRITE(6,18) I,NUMPT(I),U,V,W
230 CONTINUE
999 RETURN
END
SUBROUTINE ELTYPE(MTYPE,KGEOM)

```













6	GO TO 1000					ELER0980
	CONTINUE					ELER0990
9006	WRITE(6,9006)					ELER1000
	FORMAT(/,IX,'A3NORMAL	TERMINATION IN SCL21 ,ELEMENT	CARD	ERROR'//)		ELER1010
	GO TO 1000					ELER1020
7	CONTINUE					ELER1030
	WRITE(6,9007)					ELER1040
9007	FORMAT(/,IX,'A3NORMAL	TERMINATION IN ADTRUS,ELEMENT	CARD	ERROR'//)		ELER1050
	GO TO 1000					ELER1060
8	CONTINUE					ELER1070
	WRITE(6,9008)					ELER1080
9008	FORMAT(/,IX,'A3NORMAL	TERMINATION IN ADPLAN,ELEMENT	CARD	ERROR'//)		ELER1090
	GO TO 1000					ELER1100
9	CONTINUE					ELER1110
	WRITE(6,9009)					ELER1120
9009	FORMAT(/,IX,'A3NORMAL	TERMINATION IN AD3DEE,ELEMENT	CARD	ERROR'//)		ELER1130
	GO TO 1000					ELER1140
10	CONTINUE					ELER1150
	WRITE(6,9010)					ELER1160
9010	FORMAT(/,IX,'A3NORMAL	TERMINATION IN ADBEAM,ELEMENT	CARD	ERROR'//)		ELER1170
	GO TO 1000					ELER1180
11	CONTINUE					ELER1190
	WRITE(6,9011)					ELER1200
9011	FORMAT(/,IX,'A3NORMAL	TERMINATION IN NSTRUS,ELEMENT	CARD	ERROR'//)		ELER1210
	GO TO 1000					ELER1220
12	CONTINUE					ELER1230
	WRITE(6,9012)					ELER1240
9012	FORMAT(/,IX,'A3NORMAL	TERMINATION IN NSPLAN,ELEMENT	CARD	ERROR'//)		ELER1250
	GO TO 1000					ELER1260
13	CONTINUE					ELER1270
	WRITE(6,9013)					ELER1280
9013	FORMAT(/,IX,'A3NORMAL	TERMINATION IN NS3DEE,ELEMENT	CARD	ERROR'//)		ELER1290
	GO TO 1000					ELER1300
14	CONTINUE					ELER1310
	WRITE(6,9014)					ELER1320
9014	FORMAT(/,IX,'A3NORMAL	TERMINATION NONSAP MESH CANNOT BE PLOTTED'//)				ELER1330
	GO TO 1000					ELER1340
15	CONTINUE					ELER1350
	GO TO 1000					ELER1360
16	CONTINUE					ELER1370
	GO TO 1000					ELER1380
17	CONTINUE					ELER1390
	GO TO 1000					ELER1400
18	CONTINUE					ELER1410
	GO TO 1000					ELER1420
19	CONTINUE					ELER1430
	GO TO 1000					ELER1440
20	CONTINUE					ELER1450



```

1000 CONTINUE
      CALL PSTOP
      RETURN
      END
      SUBROUTINE GEOM9(NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C      *      *      *      *      *      *      *      *      *
C      ***      GEOM9 READS SAP IV GEOMETRY DATA
C      ***      CALLED BY PSAPI
C      *      *      *      *      *      *      *      *      *
C      COMMON/CONTRL/ KGEOM, KDATA, KPLOT, KSYMXY, KSYMXXZ, KSYMZY, XLHT,
1KHORZ, KVERT, PHI, THETA, PSI, NEWFR, ISCALE, PLOTSZ, XORGN, YORGN,
2PSCALE, KDISP, DMAG, KODE
C      COMMON/KOUNT// VNODE, NNDIST, NUDISP, NWDISP
C      COMMON/GCONT/NJMNP, NPAR(20), NELTYP, NJMEL
C      DIMENSION NUMPT(1), XPT(1), YPT(1), ZPT(1), UPT(1), VPT(1), WPT(1)
C      DATA CTEST/0, /
C      ***      INSERT ROUTINE HERE
C      *      *      *      *      *      *      *      *      *
100      READ(5,100) HED
          FORMAT(12A6)
C      *      *      *      *      *      *      *      *      *
C      ***      READ MASTER CONTROL CARD
C      ***      NUMNP = TOTAL NUMBER OF NODE POINTS
C      ***      NELTYP = NUMBER OF ELEMENT GROUPS
C      *      *      *      *      *      *      *      *      *
200      READ(5,200) NUMNP, NELTYP
          FORMAT(2I5)
          NNODE=NUMNP
C      *      *      *      *      *      *      *      *      *
C      *****READ OR GENERATE NODAL POINT DATA
C      *      *      *      *      *      *      *      *      *
          NOLD=0
10      READ(5,9006) CT,N,XPT(N),YPT(N),ZPT(N),KN
9006      FORMAT(A1,I4,30X,3F10.0,I5)
C      *      *      *      *      *      *      *      *      *
C      ***CHECK FOR CYLINDRICAL COORDINATES
C      *      *      *      *      *      *      *      *      *
          IF(CT.NE.CTEST) GO TO 20
              R=XPT(N)
              XPT(N)=R*SIN(ZPT(N)/57.2958)
              ZPT(N)=R*CS(ZPT(N)/57.2958)
C      *      *      *      *      *      *      *      *      *

```

```

ELER1460
ELER1470
ELER1480
ELER1490
SAPF0010
SAPF0020
SAPF0030
SAPF0040
SAPF0050
SAPF0060
SAPF0070
SAPF0080
SAPF0090
SAPF0100
SAPF0110
SAPF0120
SAPF0130
SAPF0140
SAPF0150
SAPF0160
SAPF0170
SAPF0180
SAPF0190
SAPF0200
SAPF0210
SAPF0220
SAPF0230
SAPF0240
SAPF0250
SAPF0260
SAPF0270
SAPF0280
SAPF0290
SAPF0300
SAPF0310
SAPF0320
SAPF0330
SAPF0340
SAPF0350
SAPF0360
SAPF0370
SAPF0380
SAPF0390
SAPF0400
SAPF0410
SAPF0420
SAPF0430
SAPF0440

```

















```

C
COMMON/GCONT/NUMNP,NPAR(20),NELTYP,NUMEL
N2=2
NUME=NPARG(2)
NUMEPC=NPARG(3)
NUMEF=NPARG(4) * 2
NUMMAT=NPARG(5)
READ MATERIAL PROPERTY CARDS (DUMMY)
DO 10 I=1,NUMMAT
  READ(5,1001) DUMMY
FORMAT(10A8)
1001 CONTINUE
10 CONTINUE
C ** READ ELEMENT PROPERTY CARDS (DUMMY1)
DO 20 J=1,NUMEPC
  READ(5,1001) DUMMY1
20 CONTINUE
C ** READ ELEMENT LOAD MULTIPLIERS(DUMMY2)
DO 30 K=1,3
  READ(5,1001) DUMMY2
30 CONTINUE
C ** READ FIXED-END FORCE CARDS(DUMMY3)
DO 40 L=1,NUMFEF
  READ(5,1001) DUMMY3
40 CONTINUE
C ** IF(NPAR(14).EQ.0) NPAR(14) = 1
N=NPARG(14)
READ ELEMENT CONNECTION INFO
100 READ(5,1002) I,I,J,J,KK
1002 FORMAT(3I5,47X,I8)
IF (KK.EQ.0) KK=1
IF (M.NE.N) GO TO 200
I = I
J = J
KKK = KK
CONTINUE
200 NUMEL = NUMEL+1
WRITE(10) N2,N,I,J
IF (N.EQ.NUMEL) RETURN
N = N + 1
I = I + KKK
J = J + KKK
IF (N.GT.M) GO TO 100
GO TO 120
END
SUBROUTINE THREEED
C
C
C

```

















C

```

DIMENSION NP(20), INP(20)
COMMON/GCONT/NJMNP,NPAR(20),NELTYP,NUMEL
N20=20
NSOL21=NP(2)
NUMMAT=NP(3)
MAXTP=NP(4)
IF(MAXTP.EQ.0) MAXTP=1
NORTHQ=NP(5)
NDLS=NP(6)
MAXNOD=NP(7)
IF(MAXNOD.EQ.0) MAXNOD=21
IF(MAXNOD.EQ.8) N20=8
NOPSET=NP(8)
READ THE MATERIAL PROPERTY CARDS
DO 50 J=1,NUMMAT
  9002 FORMAT(2I5)
  IF(NTP.EQ.0) NTP=1
  NTP2=2*NTP
  DO 40 JJ=1,NTP2
    READ(5,9004) DUMMY
  9004 FORMAT(20A4)
  CONTINUE
  40
  50
  CONTINUE
  READ MATERIAL AXES ORIENTATION SETS
  IF(NORTHQ.EQ.0) GO TO 61
  DO 60 J=1,NORTHQ
    READ(5,9004) DUMMY
  60
  CONTINUE
  61
  CONTINUE
  READ DISTRIBUTED SURFACE LOAD DATA
  IF(NDLS.EQ.0) GO TO 71
  NDLS2=NDLS*2
  DO 70 J=1,NDLS2
    READ(5,9004) DUMMY
  70
  CONTINUE
  71
  CONTINUE
  READ STRESS OUTPUT LOCATION SETS
  IF(NOPSET.EQ.0) GO TO 81
  DO 80 J=1,NOPSET
    READ(5,9004) DUMMY
  80
  CONTINUE
  81
  CONTINUE
  READ ELEMENT LOAD CASE MULTIPLIERS
  DO 90 J=1,5
    READ(5,9004) DUMMY
  90
  CONTINUE

```





```

C *** READ ELEMENT DATA CARDS
      IF(NPAR(14).EQ.0) NPAR(14)=1
      NEL=NPAR(14)-1
      130 READ(5,9006) IVEL,IINC
      9006 FORMAT(15,35X,15)
      9008 READ(5,9008) (INP(I),I=1,N20)
      9008 FORMAT(1615)
      IF(IINC.EQ.0) IINC=1
      140 NEL=NEL+1
      ML=INEL-NEL
      IF(ML) 150,155,160
      150 CALL ERROR(6)
      *** NO GENERATION OF NODE POINTS REQUIRED
      155 DO 156 I=1,N20
        NP(I)=INP(I)
        CONTINUE
      156 GO TO 162
      *** GENERATION OF NODE POINTS REQUIRED
      160 DO 161 I=1,N20
        IF(NP(I).EQ.0) GO TO 161
        NP(I)=NP(I)+KN
        CONTINUE
      161 CONTINUE
      162 NUMEL=NUMEL+1
      WRITE(10) N20,VEL,(NP(I),I=1,N20)
      IF(NEL.EQ.NSOL21) RETURN
      IF(NEL.LT.INEL) GO TO 140
      KN=IINC
      GO TO 130
      END
      SUBROUTINE GEOM1(NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C * * * * *
C *** THIS ROUTINE READS ADINA DATA CARDS FROM THE TITLE CARD TO THE
C *** ELEMENT CONTROL CARDS - IT IS CALLED BY PSAPI
C * * * * *
      COMMON/CONTROL/ KGEOM,KDATA,KPLOT,KSYMXY,KSYMxz,KSYMZY,NOTAT,XLHT,
      1KHORZ,KVERT,PHI,THETA,PSI,NEWFR,ISCALE,PLOTSZ,XORGN,YORGN,
      2PSCALE,KDISP,DWAG,KODE
      COMMON/KOUNT/ NNODE,NNEST,NUDISP,NWDISP
      COMMON/GCONT/NUMNP,NPAR(20),NELTYP,NUMEL
      DIMENSION NUMPT(1),XPT(1),YPT(1),ZPT(1),UPT(1),VPT(1),WPT(1)
      DIMENSION IDOF(6),ID(6),IDOLD(6)
      1,NODE(20)
      DATA CTEST/'X' /

```

```

SAPF4290
SAPF4300
SAPF4310
SAPF4320
SAPF4330
SAPF4340
SAPF4350
SAPF4360
SAPF4370
SAPF4380
SAPF4390
SAPF4400
SAPF4410
SAPF4420
SAPF4430
SAPF4440
SAPF4450
SAPF4460
SAPF4470
SAPF4480
SAPF4490
SAPF4500
SAPF4510
SAPF4520
SAPF4530
SAPF4540
SAPF4550
SAPF4560
SAPF4570
SAPF4580
ADNA0010
ADNA0020
ADNA0030
ADNA0040
ADNA0050
ADNA0060
ADNA0070
ADNA0080
ADNA0090
ADNA0100
ADNA0110
ADNA0120
ADNA0130
ADNA0140
ADNA0150
ADNA0160
ADNA0170
ADNA0180

```



```

NCARD=0
READ(5, 9000) DUMMY
FORMAT(20A4)
C *** READ MASTER CONTROL CARDS
C *** NUMNP = TOTAL NUMBER OF NODE POINTS
C *** NELTYP = NUMBER OF ELEMENT GROUPS
9001 READ(5, 9001) NUMNP, (IDOF(I), I=1, 6), NEGL, NEGNL, MODEX, NSTE
FORMAT(I5, 6I1, I4, 3I5)
NELTYP=NEGL+NEGNL
NNODE=NUMNP
READ(5, 9002) IMASS, IDAMP, IMASSN, IDAMPN
FORMAT(4I5)
9002 READ(5, 9002) IEIG
READ(5, 9002) ISREF, NUMREF, IEQUIT, ITEMAX
READ(5, 9000) DUMMY
READ(5, 9000) DUMMY
READ(5, 9000) DUMMY
READ(5, 9000) DUMMY
C *** READ OR GENERATE NODAL POINT DATA
NOLD=0
NEQ=0
10 READ(5, 9006) CT, N, (ID(I), I=1, 6), XPT(N), YPT(N), ZPT(N), KN
9006 FORMAT(A1, I4, I4, I4, 5I5, 3F10, 0, I5)
C *** CHECK FOR CYLINDRICAL COORDINATES
IF(CT.NE.CTEST) GO TO 12
DUM=ZPT(N)/57.2958
R=YPT(N)
YPT(N)=R*COS(ZPT(N)/57.2958)
ZPT(N)=R*SIN(ZPT(N)/57.2958)
12 CONTINUE
NUMPT(N)=N
IF(NOLD.EQ.0) GO TO 50
FOR GENERATION OF FIXED BOUNDARY CONDITIONS
DO 15 I=1, 6
IF(IDOLD(I).EQ.-1.AND.ID(I).EQ.0) ID(I)=IDOLD(I)
15 CONTINUE
IF(KNOLD.EQ.0) GO TO 50
NUM=(N-NOLD)/KNOLD
NUMN=NUM-1
IF(NUMV.LT.1) GO TO 50
TO COUNT DOFS TO DETERMINE NUMBER OF IC CARDS
DO 20 I=1, 6
IF(IDOF(I).EQ.0.AND.IDOLD(I).EQ.0) NEQ=NEQ+NUMN
20 CONTINUE
DX=(XPT(N)-XPT(NOLD))/NUM
IF(CT.NE.CTEST) GO TO 21
ROLD=YPT(NOLD)/COS(DUMOLD)
RNEW=YPT(N)/COS(DUM)
DR=(RNEW-ROLD)/NUM

```

```

ADNA0190
ADNA0200
ADNA0210
ADNA0220
ADNA0230
ADNA0240
ADNA0250
ADNA0260
ADNA0270
ADNA0280
ADNA0290
ADNA0300
ADNA0310
ADNA0320
ADNA0330
ADNA0340
ADNA0350
ADNA0360
ADNA0370
ADNA0380
ADNA0390
ADNA0400
ADNA0410
ADNA0420
ADNA0430
ADNA0440
ADNA0450
ADNA0460
ADNA0470
ADNA0480
ADNA0490
ADNA0500
ADNA0510
ADNA0520
ADNA0530
ADNA0540
ADNA0550
ADNA0560
ADNA0570
ADNA0580
ADNA0590
ADNA0600
ADNA0610
ADNA0620
ADNA0630
ADNA0640
ADNA0650
ADNA0660

```



```

DT=(DUM-DUMOLD)/NUM
GO TO 22
21 CONTINUE
DY=(YPT(N)-YPT(NOLD))/NUM
DZ=(ZPT(N)-ZPT(NOLD))/NUM
22 CONTINUE
K=NOLD
DO 30 J=1,NUMN
  KK=K
  K=K+KNOLD
  XPT(K)=XPT(KK)+DX
  IF(CT.NE.CTEST) GO TO 26
  ROLD=ROLD+DR
  DUMOLD=DUMOLD+DT
  YPT(K)=ROLD*COS(DUMOLD)
  ZPT(K)=ROLD*SIN(DUMOLD)
  GO TO 28
26 CONTINUE
  YPT(K)=YPT(KK)+DY
  ZPT(K)=ZPT(KK)+DZ
  CONTINUE
28 CONTINUE
  NUMPT(K)=K
  CONTINUE
30 CONTINUE
50 NOLD=N
  KNOLD=KN
  DUMOLD=DUM
  TO COUNT DOFS TO DETERMINE NUMBER OF IC CARDS
  DO 55 I=1,6
    IF(IDOF(I).EQ.0.AND.ID(I).EQ.0) NEQ=NEQ+1
    IDOLD(I)=ID(I)
  CONTINUE
55 CONTINUE
  IF(N.NE.NUMNP) GO TO 10
  READ LOAD CONTROL CARDS
  READ(5,9000) DJMMY
  DO 80 I=1,IMASSN
    IF(IMASSN.EQ.0) GO TO 81
    READ(5,9000) DUMMY
  CONTINUE
80 CONTINUE
81 CONTINUE
  IF(IDAMPN.EQ.0) GO TO 91
  DO 90 I=1,IDAMPN
    READ(5,9000) DUMMY
  CONTINUE
90 CONTINUE
91 CONTINUE
  READ INITIAL CONDITIONS
  READ(5,9002) ICON
  IF(ICON.EQ.0) GO TO 100
  CARDNR=NEQ/6.0

```



```

NCARD=INT(CARDNR)
TEST=CARDNR-NCARD
IF(TEST.GT.0.1) NCARD=NCARD+1
DO 95 I=1,NCARD
  READ(5,9000) DUMMY
  CONTINUE
95 IF(IMASS.EQ.0) GO TO 100
DO 96 I=1,NCARD
  READ(5,9000) DUMMY
  CONTINUE
96 DO 98 I=1,NCARD
  READ(5,9000) DUMMY
  CONTINUE
98 FORMAT(6E12.6)
9007 CONTINUE
100 NUMEL=0
WRITE(6,9009) NEQ,NCARD
9009 FORMAT(///,' NEQ AND NCARD FOR IC IN GEOM1 = ',I5,I0X,I5,///)
C *** READ ELEMENT CONTROL CARDS
DO 900 M=1,NELTYP
  READ(5,9010) (NPAR(I),I=1,20)
  WRITE(6,9010) (NPAR(I),I=1,20)
9008 FORMAT(20I4)
9010 FORMAT(///,' NPAR = ',20I5,///)
      MTYPE=NPAP(1)
      CALL ELTYPE(MTYPE,KGEOM)
900 CONTINUE
      ENDFILE 10
999 RETURN
END
SUBROUTINE ADTRUS
C * * * * *
C *** THIS SUBROUTINE TO READ ADINA TRUSS DATA
C *** THIS ROUTINE CALLED BY ELTYPE
C * * * * *
COMMON/GCONT/NUMNP,NPAR(20),NELTYP,NUMEL
NUMMAT=NPAP(16)
N2=2
IF(NUMMAT.EQ.0) NUMMAT=1
IF(NPAR(15).EQ.1) NCARD=2
IF(NPAR(15).EQ.3) NCARD=3
IF(NPAR(15).NE.2) GO TO 20
CARDNR=NPAP(17)/8.0
NCARD=INT(CARDNR)
ADNA1150
ADNA1160
ADNA1170
ADNA1180
ADNA1190
ADNA1200
ADNA1210
ADNA1220
ADNA1230
ADNA1240
ADNA1250
ADNA1260
ADNA1270
ADNA1280
ADNA1290
ADNA1300
ADNA1310
ADNA1320
ADNA1330
ADNA1340
ADNA1350
ADNA1360
ADNA1370
ADNA1380
ADNA1390
ADNA1400
ADNA1410
ADNA1420
ADNA1430
ADNA1440
ADNA1450
ADNA1460
ADNA1470
ADNA1480
ADNA1490
ADNA1500
ADNA1510
ADNA1520
ADNA1530
ADNA1540
ADNA1550
ADNA1560
ADNA1570
ADNA1580
ADNA1590
ADNA1600
ADNA1610
ADNA1620

```











```

C ***
NUMMAT=NPARG(16)
NSTRES=NPARG(13)
CALCULATE THE NUMBER OF MATERIAL CASE CARDS
IF(NPARG(15).EQ.1) NCARD=1
IF(NPARG(15).EQ.2) NCARD=2
IF(NPARG(15).EQ.3) NCARD=3
IF(NPARG(15).EQ.4) NCARD=4
IF(NPARG(15).EQ.5) NCARD=5
IF(NPARG(15).EQ.6) NCARD=6
IF(NPARG(15).EQ.7) NCARD=7
IF(NPARG(15).EQ.8) NCARD=8
IF(NPARG(15).EQ.9) NCARD=9
IF(NPARG(15).EQ.10) NCARD=10
IF(NPARG(15).EQ.11) NCARD=11
IF(NPARG(15).EQ.12) NCARD=12
IF(NPARG(15).EQ.13) NCARD=13
IF(NPARG(15).EQ.14) NCARD=14
CARDNR=NPARG(17)/8.0
NCARD=INT(CARDNR)
TEST=CARDNR-NCARD
IF(TEST.GT.0.1) NCARD=NCARD+1
20 CONTINUE
N12=12
C ***
READ MATERIAL PROPERTIES
DO 50 J=1,NUMMAT
READ(5,9000) DUMMY
9000 FORMAT(20A4)
DO 45 I=1,NCARD
READ(5,9000) DUMMY
45 CONTINUE
50 CONTINUE
C ***
CONTINUE OUTPUT TABLE CARDS
READ STRESS OUTPUT TABLE CARDS
IF(NPARG(13).EQ.0) GO TO 61
DO 60 I=1,NSTRES
READ(5,9000) DUMMY
60 CONTINUE
61 CONTINUE
C ***
CONTINUE AND GENERATE ELEMENT DATA CARDS
IF(NPARG(14).EQ.0) NPARG(14)=1
NEL=NPARG(14)-1
READ(5,9002) IVEL,IINC
IF(IINC.EQ.0) IINC=1
9002 FORMAT(15,15X,15)
READ(5,9004) (IVP(I),I=1,8)
9004 FORMAT(8I5)
NEL=NEL+1
ML=INEL-NEL
IF(ML)150,155,160
150 CALL ERROR(8)
C ***
NO GENERATION OF NODE POINTS REQUIRED

```

```

ADNA2110
ADNA2120
ADNA2130
ADNA2140
ADNA2150
ADNA2160
ADNA2170
ADNA2180
ADNA2190
ADNA2200
ADNA2210
ADNA2220
ADNA2230
ADNA2240
ADNA2250
ADNA2260
ADNA2270
ADNA2280
ADNA2290
ADNA2300
ADNA2310
ADNA2320
ADNA2330
ADNA2340
ADNA2350
ADNA2360
ADNA2370
ADNA2380
ADNA2390
ADNA2400
ADNA2410
ADNA2420
ADNA2430
ADNA2440
ADNA2450
ADNA2460
ADNA2470
ADNA2480
ADNA2490
ADNA2500
ADNA2510
ADNA2520
ADNA2530
ADNA2540
ADNA2550
ADNA2560
ADNA2570
ADNA2580

```



```

155 DO 156 I=1,4
156 I5=I+4
156 I9=I+8
156 NP(I)=INP(I)
156 NP(I5)=0
156 NP(I9)=INP(I5)
156 CONTINUE
156 GO TO 162
C *** GENERATION OF NODE POINTS REQUIRED
160 DO 161 I=1,N12
161 IF(NP(I).EQ.0) GO TO 161
161 NP(I)=NP(I)+KN
161 CONTINUE
162 CONTINUE
162 NUMEL=NUMEL+1
162 WRITE(10) N12,NEL,(NP(I),I=1,N12)
162 IF(NEL.EQ.NPAR(2)) RETURN
162 IF(NEL.LT.INEL) GO TO 140
162 KN=IINC
162 GO TO 130
162 END
C SUBROUTINE AD3DEE
C * * * * *
C *** THIS SUBROUTINE TO READ ADINA 3-D SOLID ELEMENT DATA
C *** THIS ROUTINE CALLED BY ELTYPE
C * * * * *
C COMMON/GCONT/VUMNP,NPAR(20),NELTYP,NUMEL
C DIMENSION NP(20),INP(20)
C NUMMAT=NP(16)
C NSTRES=NP(13)
C *** CALCULATE THE NUMBER OF MATERIAL CASE CARDS
C IF(NPAR(15).EQ.1) NCARD=1
C IF(NPAR(15).EQ.2) NCARD=2+NP(18)
C IF(NPAR(15).EQ.3) NCARD=4
C IF(NPAR(15).EQ.4) NCARD=4
C IF(NPAR(15).EQ.5) NCARD=2
C IF(NPAR(15).EQ.8) NCARD=1
C IF(NPAR(15).EQ.9) NCARD=1
C IF(NPAR(15).EQ.10) NCARD=6
C IF(NPAR(15).EQ.11) NCARD=6
C IF(NPAR(15).EQ.12) GO TO 20
C CARDNR=NP(17)/8.0
C NCARD=INT(CARDNR)
C TEST=CARDNR-NCARD
ADNA2590
ADNA2600
ADNA2610
ADNA2620
ADNA2630
ADNA2640
ADNA2650
ADNA2660
ADNA2670
ADNA2680
ADNA2690
ADNA2700
ADNA2710
ADNA2720
ADNA2730
ADNA2740
ADNA2750
ADNA2760
ADNA2770
ADNA2780
ADNA2790
ADNA2800
ADNA2810
ADNA2820
ADNA2830
ADNA2840
ADNA2850
ADNA2860
ADNA2870
ADNA2880
ADNA2890
ADNA2900
ADNA2910
ADNA2920
ADNA2930
ADNA2940
ADNA2950
ADNA2960
ADNA2970
ADNA2980
ADNA2990
ADNA3000
ADNA3010
ADNA3020
ADNA3030
ADNA3040
ADNA3050
ADNA3060

```



```

      IF(TEST.GT.0.1) NCARD=NCARD+1
      20 CONTINUE
      C *** N20=20 MATERIAL PROPERTIES
      DO 50 J=1,NUMMAT
      9000 READ(5,9000) DUMMY
      DO 45 I=1,NCARD
      45 READ(5,9000) DUMMY
      CONTINUE
      C *** CONTINUE OUTPUT TABLE CARDS
      IF(NPAR(13).EQ.0) GO TO 61
      DO 60 I=1,NSTRES
      60 READ(5,9000) DUMMY
      CONTINUE
      61 CONTINUE
      IF(NPAR(14).EQ.0) NPAR(14)=1
      NEL=NPAR(14)-1
      130 READ(5,9002) INEL,IINC
      9002 FORMAT(15,30X,I5)
      IF(IINC.EQ.0) IINC=1
      READ(5,9004) (INP(I),I=1,8)
      9004 READ(5,9004) (INP(I),I=9,N20)
      140 FORMAT(12I5)
      NEL=NEL+1
      ML=INEL-NEL
      IF(ML) 150,155,160
      150 CALL ERROR(9)
      C *** NO GENERATION OF NODE POINTS REQUIRED
      155 DO 156 I=1,N20
      156 NP(I)=INP(I)
      CONTINUE
      GO TO 162
      C *** GENERATION OF NODE POINTS REQUIRED
      160 DO 161 I=1,N20
      161 IF(NP(I).EQ.0) GO TO 161
      NP(I)=NP(I)+KN
      CONTINUE
      161 CONTINUE
      162 NUMEL=NUMEL+1
      WRITE(10) N20,NEL,(NP(I),I=1,N20)
      IF(NEL.EQ.NPAR(2)) RETURN
      IF(NEL.LT.INEL) GO TO 140
      KN=IINC
      GO TO 130
      END
      SUBROUTINE AD3EAM

```

ADNA3070  
ADNA3080  
ADNA3090  
ADNA3100  
ADNA3110  
ADNA3120  
ADNA3130  
ADNA3140  
ADNA3150  
ADNA3160  
ADNA3170  
ADNA3180  
ADNA3190  
ADNA3200  
ADNA3210  
ADNA3220  
ADNA3230  
ADNA3240  
ADNA3250  
ADNA3260  
ADNA3270  
ADNA3280  
ADNA3290  
ADNA3300  
ADNA3310  
ADNA3320  
ADNA3330  
ADNA3340  
ADNA3350  
ADNA3360  
ADNA3370  
ADNA3380  
ADNA3390  
ADNA3400  
ADNA3410  
ADNA3420  
ADNA3430  
ADNA3440  
ADNA3450  
ADNA3460  
ADNA3470  
ADNA3480  
ADNA3490  
ADNA3500  
ADNA3510  
ADNA3520  
ADNA3530  
ADNA3540









```

162 J=J+KN
      CONTINUE
      NUMEL=NUMEL+1
      WRITE(10) N2,VEL,I,J
      IF(NEL.EQ.NPAR(2)) RETURN
      IF(NEL.LT.INEL) GO TO 140
      KN=IINC
      GO TO 130
      END

```

```

ADNA4030
ADNA4040
ADNA4050
ADNA4060
ADNA4070
ADNA4080
ADNA4090
ADNA4100
ADNA4110

```



```

C ***** SUBROUTINES NFRAME AND CCRT2 DELETED *****
C SUBROUTINE GEOM2(NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C
C * * * * *
C ***** USER SUPPLIED SUBROUTINE SPACE *****
C
C * * * * *
C CALL ERROR(14)
C RETURN
C END
C SUBROUTINE NSTRUS
C
C * * * * *
C ***** THIS SUBROUTINE TO READ NON SAP TRUSS ELEMENTS
C ***** CALLED BY ELTYPE *****
C
C * * * * *
C RETURN
C END
C SUBROUTINE NSPLAN
C
C * * * * *
C ***** THIS SUBROUTINE TO READ NON SAP 2 D 8 NODE PLANE ELEMENTS
C ***** CALLED BY ELTYPE *****
C
C * * * * *
C RETURN
C END
C SUBROUTINE NS3DEE
C
C * * * * *
C ***** THIS SUBROUTINE TO READ NON SAP 3-D ELEMENT DATA
C ***** CALLED BY ELTYPE *****
C
C * * * * *
C RETURN
C END
C SUBROUTINE DATA1(NUMPT,XPT,YPT,ZPT,UPT,VPT,WPT)
C
C * * * * *

```

```

APR1981
AUXL0080
APR1981
*APR1981
APR1981
APR1981
*APR1981
*APR1981
AUXL0090
AUXL0100
AUXL0110
AUXL0120
AUXL0130
*APR1981
AUXL0140
AUXL0150
AUXL0160
AUXL0170
AUXL0180
*APR1981
AUXL0190
AUXL0200
AUXL0210
AUXL0220
AUXL0230
AUXL0240
*APR1981
AUXL0250
AUXL0260
AUXL0270
AUXL0280
AUXL0290
*APR1981
AUXL0300
AUXL0310
AUXL0320
AUXL0330
AUXL0340
AUXL0350
*APR1981
AUXL0360
AUXL0370
AUXL0380
AUXL0390
AUXL0400
*APR1981
AUXL0410
AUXL0420
AUXL0430
AUXL0440
AUXL0450
AUXL0460
*APR1981
APR1981

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